

DESIGN VALUES:			DESIGN VALUES ¹
DEAD AND LIVE LOADS			
ROOF LIVE LOAD		20 PSF	
ROOF DEAD LOAD (SUPERIMPOSED ON FRAME) ²		LOAD SCENARIO=(1,2) DL=(3.5 PSF, 2.0 PSF)	
ALLOWABLE SOIL PRESSURE^{3,5}			
SPREAD PAD			
VERTICAL BEARING: DL + Lr + SEISMIC (CONCRETE FOOTING)		1500 PSF	
LATERAL COHESION: DL + Lr + SEISMIC (CONCRETE FOOTING)		130 PSF	
DRILLED PIER			
SKIN FRICTION (DOWN): DL + Lr + SEISMIC (CONCRETE FOOTING) PER 1810A.3.3.1.4		167 PSF	
SKIN FRICTION (UPLIFT): DL + Lr + SEISMIC (CONCRETE FOOTING) PER 1810A.3.3.1.5		83 PSF	
LATERAL BEARING: DL + Lr + SEISMIC (CONCRETE FOOTING) PER 1810A.3.3.2		100 PSF/FT	
ROOF SNOW LOAD⁶			
GROUND SNOW LOAD, Pg		10 PSF	
RISK CATEGORY		III	
ROOF SNOW LOAD: [] FLAT, Pf OR [] LOW SLOPE, Pm OR [X] SLOPED, Ps		11 PSF	
SNOW ROOF SLOPE FACTOR, Cs		1.0	
SNOW EXPOSURE FACTOR, Ce		1.2	
SNOW LOAD IMPORTANCE FACTOR, Is		1.1	
THERMAL FACTOR, Ct		1.2	
DRIFT SURCHARGE LOAD, Pd		0 PSF	
DISTANCE FROM ADJACENT STRUCTURE, Pg = 0 PSF		4 IN	
DISTANCE FROM ADJACENT STRUCTURE, Pg > 0 PSF		20 FT	
ICE LOAD		0 PSF	
FLOOD DESIGN			
FLOOD HAZARD AREA		NO	
WIND DESIGN⁴			
BASIC WIND SPEED (3 SECOND GUST), Vult		115 MPH	
EXPOSURE CATEGORY		C	
TOPOGRAPHIC FACTOR, Kzf (1 MINIMUM)		1	
INTERNAL PRESSURE COEFFICIENT, GCpi (IF APPLICABLE)		0.0	
CLEAR WIND FLOW		YES	
OBSTRUCTED WIND FLOW		YES	
SEISMIC DESIGN⁴			
LATERAL FORCE-RESISTING SYSTEM		STEEL ORDINARY CANTILEVER COLUMN SYSTEM	
ANALYSIS PROCEDURE		EQUIVALENT LATERAL FORCE PROCEDURE	
SEISMIC DESIGN CATEGORY (SDC)		E	
SEISMIC IMPORTANCE FACTOR, Ie		1.25	
DESIGN BASE SHEAR, V		Cs x W	
SEISMIC RESPONSE COEFFICIENT, Cs		LOAD SCENARIO = (1,2) Cs = (1.13, 1.65)	
RESPONSE MODIFICATION FACTOR, R		1.25	
SITE CLASS ⁷		E	
REDUNDANCY FACTOR, p		1.3	
MAPPED SPECTRAL RESPONSE ACCELERATION AT SHORT PERIOD, Ss - USED TO DETERMINE Cs		LOAD SCENARIO = (1,2) Ss = (1.406, 2.063)	
SHORT-PERIOD SITE COEFFICIENT, Fa		1.2	
DESIGN SPECTRAL RESPONSE ACCELERATION AT SHORT PERIOD, Sds - USED TO DETERMINE Cs		LOAD SCENARIO = (1,2) Sds (MAX) = (1.125, 1.650)	
MAPPED SPECTRAL RESPONSE ACCELERATION AT 1 SECOND PERIOD, S1		LOAD SCENARIO = (1,2) S1 = (0.844, 1.07)	
LONG-PERIOD SITE COEFFICIENT, Fv		2.0	
DESIGN SPECTRAL RESPONSE ACCELERATION AT 1 SECOND PERIOD, Sd1		LOAD SCENARIO = (1,2) Sd1 = (1.125, 1.427)	
HORIZONTAL OR VERTICAL IRREGULARITIES TYPE(S)		NONE	

- IF SITE-SPECIFIC DESIGN CRITERIA ARE OUTSIDE THE LIMITS OF THESE PC DRAWINGS, CONTACT POLYGON ENGINEERING TO SEE IF AN ENGINEERING LETTER, SUPPLEMENTAL DRAWINGS, AND/OR CALCULATIONS COULD BE SUBMITTED FOR A SITE-SPECIFIC SOLUTION. ANY SITE-SPECIFIC DEVIATION FROM THIS PC MAY NOT BE SUBMITTED TO DSA AS AN OVER-THE-COUNTER
- STRUCTURE IS NOT DESIGNED TO SUPPORT SOLAR PANELS. STRUCTURE IS NOT DESIGNED TO SUPPORT SPRINKLER SYSTEMS IN LOAD SCENARIO 2 REGIONS.
- GEHAZARD REPORTS ARE REQUIRED IF THE AREA COVERED UNDER THE ROOF EXCEEDS 4000 SQ FT OR IS LOCATED WITHIN STATE OR LOCAL GEOLOGIC HAZARD ZONE. VERIFY SUBMITAL AND APPROVAL OF A GEHAZARD REPORT BY CGS PRIOR TO DSA SITE APPLICATION.
- STRUCTURAL SEPARATION BETWEEN ADJACENT STRUCTURES: CWC10= 2.75" CWC15= 3.00" STRUCTURAL SEPARATION BETWEEN EXISTING STRUCTURES: CWC10= 3.75" CWC15= 4.00"
- WHEN PLACING MULTIPLE CANOPIES WITH PIER FOOTINGS ADJACENT TO ONE ANOTHER, THE DESIGN MAY REQUIRE AN ANALYSIS OF GROUP EFFECTS ON THE FOUNDATIONS, THE MINIMUM CLEARANCE BETWEEN CENTER OF PIERS IS EIGHT TIMES PIER DIAMETER WITHOUT AN ACCOMPANYING ENGINEERING LETTER.
- SITE APPLICATION DESIGN PROFESSIONAL AND DSA REVIEWER SHALL VERIFY THE STRUCTURE TO BE LOCATED AT LEAST 20 FEET FROM ANY ADJACENT HIGHER STRUCTURE IF GROUND SNOW LOAD IS GREATER THAN ZERO.
- DESIGN COMPLIES WITH THE CONDITIONS OF EXCEPTION 1 OF ASCE 7-16 SECTION 11.4.8, ITEM 2.
- APPROVED FIRE APPARATUS ACCESS ROADS SHALL EXTEND TO WITHIN 150 FEET OF ALL PORTIONS OF THE PERIMETER OF THE STRUCTURE PER CFC 503.1.1.

ARCHITECTURAL REQUIREMENTS:

DESCRIPTION	DESIGN VALUES
TYPE OF CONSTRUCTION	II B
NUMBER OF STORIES	1
FIRE SPRINKLER SYSTEM	NOT BY POLYGON

RELATED BUILDING CODES AND STANDARDS:

TITLE 24 CODES:
2022 California Administrative Code (CAC) (Part 1, Title 24, CCR)
2022 California Building Code (CBC), Volumes **1 and 2** (Part 2, Title 24, CCR) (2021 International Building Code with 2022 California amendments)
2022 California Electrical Code (Part 3, Title 24, CCR)
2022 California Fire Code (CFC) (Part 4, Title 24, CCR)
2022 California Mechanical Code (CMC) (Part 4, Title 24, CCR) (2021 Uniform Mechanical Code with 2022 California amendments)
2022 California Plumbing Code (CPC) (Part 5, Title 24, CCR) (2021 Uniform Plumbing Code with 2022 California amendments)
2022 California Energy Code (Part 6, Title 24, CCR)
2022 California Fire Code (CFC) (Part 9, Title 24, CCR) (2021 International Fire Code with 2022 California Amendments)
2022 California Existing Building Code (CEBC) (Part 10, Title 24, CCR)
2022 California Green Building Standards Code (Part 11, Title 24, CCR)
2022 California Referenced Standards Code (Part 12, Title 24, CCR)
Title 19 CCR, Public Safety, State Fire Marshal Regulations
NFPA 13 - 2022
NFPA 72 - 2022

REFERENCE CODE SECTIONS FOR APPLICABLE STANDARDS:

2022 CBC, CHAPTER 35
2022 CFC, CHAPTER 80
SCOPE OF WORK NARRATIVE:
 THESE DRAWINGS ILLUSTRATE THE FABRICATION AND INSTALLATION REQUIREMENTS FOR A FREE-STANDING PREFABRICATED STEEL SHADE STRUCTURE. THE ENTIRE STRUCTURAL SYSTEM IS COMPRISED OF TUBULAR STEEL MEMBERS SUPPORTED ON CONCRETE FOUNDATIONS. THE FLEXIBILITY INCLUDED HEREIN ALLOWS THIS STRUCTURE TO COMPLY WITH A WIDE VARIETY OF PROJECT SITES AND LOADING REQUIREMENTS.

INSTRUCTIONS FOR ARCHITECTS PLANNING TO SUBMIT THESE PRE-CHECKED DRAWINGS TO DSA:

STEP 1 PROJECT INFORMATION

PROJECT NAME		
SCHOOL DISTRICT		
USE AND OCCUPANCY CLASSIFICATION	(PROPOSED OCCUPANCY: A1, A2, A3, A4, A5, B, E)	
OCCUPANT LOAD FACTOR	(15 SQFT/PERSON MAX; 5 SQFT/PERSON MIN FOR ANY A OCCUPANCY 20 SQFT/PERSON MAX FOR B or E OCCUPANCY)	
TOTAL ROOF AREA		
NUMBER OF OCCUPANTS		

STEP 2 DESIGN OPTIONS			
ROOF DECK	[] MULTI-RIB (MR)	DEFAULT	WEIGHT 1.2 PSF
	[] STANDING SEAM (SS)		WEIGHT 1.8 PSF
GUTTERS	[] NO	DEFAULT	
	[] YES		SEE CWC7.0 FOR DETAILS
ELECTRICAL ACCESS	[] NO	DEFAULT	
	[] YES		SEE CWC7.1 FOR DETAILS
CLEAR HEIGHT	[] 8'	DEFAULT	
	[] OTHER		10' MAX

STEP 3 SEISMIC ACCELERATION			
\$s		(g)	
\$1		(g)	

STEP 4 SEISMIC REGIONS			
0.000 <\$s <= 1.406	\$1 <= 0.844	[] WHITE	3.5 PSF MAX DEAD LOAD
1.406 <\$s <= 2.063	\$1 <= 1.070	[] GREEN	2.0 PSF MAX DEAD LOAD

STEP 5 TOTAL ROOF DEAD LOAD			
ROOF DECK	_____ PSF		SEE STEP 2 'ROOF DECK' FOR WEIGHTS
COLLATERAL	_____ PSF		LIGHTING , FIRE SUPPRESSION, ETC.
TOTAL	_____ PSF		ADD 'ROOF DECK' AND 'COLLATERAL'

STEP 6 LOAD SCENARIO			
WHITE	TOTAL ROOF DEAD LOAD <= 3.5 PSF	[]	LOAD SCENARIO 1
GREEN	TOTAL ROOF DEAD LOAD < 2.0 PSF	[]	LOAD SCENARIO 2

STEP 7 PC STRUCTURE			
	ROOF WIDTH <= 10	[]	CWC 10
	10 < ROOF WIDTH <= 15	[]	CWC 15

STEP 8 STRUCTURE SIZE						
	CWC 10			CWC 15		
ROOF WIDTH	[] 10'	DEFAULT	[] 15'	DEFAULT		
	[] OTHER	6' MIN; 10' MAX	[] OTHER	10'-6' MIN; 15' MAX		
ROOF LENGTH	[] 44'	2 BAYS	[] 36'	2 BAYS		
	[] 64'	3 BAYS	[] 52'	3 BAYS		
	[] 84'	4 BAYS	[] 68'	4 BAYS		
	[] OTHER		[] OTHER			

STEP 9 FOUNDATION TYPE						
FOUNDATION TYPE	CWC 10			CWC 15		
	[] SPREAD PAD	[] DRILLED PIER	[] SPREAD PAD	[] DRILLED PIER		

STEP 10 FOUNDATION SUMMARY								
	CWC 10				CWC 15			
	[] LOAD SCENARIO 1 SPREAD PAD	[] LOAD SCENARIO 1 DRILLED PIER	[] LOAD SCENARIO 1 SPREAD PAD	[] LOAD SCENARIO 1 DRILLED PIER				
	[] LOAD SCENARIO 2 SPREAD PAD	[] LOAD SCENARIO 2 DRILLED PIER	[] LOAD SCENARIO 2 SPREAD PAD	[] LOAD SCENARIO 2 DRILLED PIER				

STEP 11 SHEET INDEX								
BASE FRAME ROOF DECK	CWC 10 SHEET INDEX				CWC 15 SHEET INDEX			
	MR	SS	MR	SS	MR	SS	MR	SS
FOUNDATION TYPE	SPREAD PAD	DRILLED PIER	SPREAD PAD	DRILLED PIER	SPREAD PAD	DRILLED PIER	SPREAD PAD	DRILLED PIER
SELECT ONE	[]	[]	[]	[]	[]	[]	[]	[]
ORDER FORM	CWC1.0	CWC1.0	CWC1.0	CWC1.0	CWC1.0	CWC1.0	CWC1.0	CWC1.0
NOTES AND SPECIAL INSPECTIONS	CWC1.1	CWC1.1	CWC1.1	CWC1.1	CWC1.1	CWC1.1	CWC1.1	CWC1.1
FOUNDATION PLAN	CWC2.0	CWC2.1	CWC2.0	CWC2.1	CWC2.2	CWC2.3	CWC2.2	CWC2.3
FRAMING PLAN	CWC3.0	CWC3.0	CWC3.0	CWC3.1	CWC3.1	CWC3.1	CWC3.1	CWC3.1
FRAME CONNECTION DETAILS	CWC4.0	CWC4.0	CWC4.0	CWC4.0	CWC4.1	CWC4.1	CWC4.1	CWC4.1
ARCHITECTURAL VIEWS	CWC5.0	CWC5.0	CWC5.0	CWC5.0	CWC5.1	CWC5.1	CWC5.1	CWC5.1
ROOF CONNECTION DETAILS	CWC6.0	CWC6.0	CWC6.1	CWC6.1	CWC6.0	CWC6.0	CWC6.1	CWC6.1
MISC DESIGN OPTIONS	CWC7.0	CWC7.0	CWC7.0	CWC7.1	CWC7.0	CWC7.0	CWC7.0	CWC7.0
ELECTRICAL CUTOUPS	CWC7.1	CWC7.1	CWC7.1	CWC7.1	CWC7.1	CWC7.1	CWC7.1	CWC7.1

STEP 12 MULTIPLE STRUCTURES		
	ROOF WIDTH X LENGTH	QTY
MULTIPLE STRUCTURES		

STEP 1: GENERAL PROJECT INFORMATION

- IDENTIFY PROJECT NAME AND SCHOOL DISTRICT
- IDENTIFY USE AND OCCUPANCY CLASSIFICATION
- THE USE AND OCCUPANCY DETERMINE THE MAXIMUM SQUARE FOOTAGE OF THE STRUCTURE
- THE MAXIMUM SQUARE FOOTAGE IS ALSO LIMITED BY THE NUMBER OF OCCUPANTS
- IDENTIFY THE OCCUPANT LOAD PER TABLE 1004.5 IN THE CBC
- IDENTIFY TOTAL ROOF AREA WHICH SHALL NOT EXCEED ALLOWABLE AREA PER TABLE 506.2 IN THE CBC.
- IDENTIFY EXPECTED NUMBER OF OCCUPANTS BASED ON THE ESTIMATED OCCUPANT LOAD
- TOTAL ROOF AREA DIVIDED BY OCCUPANT LOAD CAN DETERMINE NUMBER OF OCCUPANTS

STEP 2: DESIGN OPTIONS

- SELECT ROOF DECK FOR YOUR PROJECT
 - "MR" REPRESENTS MCELROY METAL "MULTI-RIB" ROOF DECK
 - "SS" REPRESENTS MCELROY METAL "MEDALLION-LOK" 16" STANDING SEAM ROOF DECK
 - SELECT WHETHER GUTTERS AND DOWNSPOUTS FROM POLYGON IS NEEDED FOR YOUR PROJECT
 - IF YES, THEN INCLUDE SHEET CWC7.0 IN THE DRAWING SET
 - SELECT WHETHER ELECTRICAL CUTOUPS ARE NEEDED FOR YOUR PROJECT
 - SHEET CWC7.0 SHOWS ELECTRICAL CUTOUP SIZE AND LOCATION CUTOUPS IN COLUMNS
 - SHEET CWC7.1 HAS INSTRUCTIONS AND SHEET TO IDENTIFY WHICH COLUMNS
 - SHEET CWC7.1 MUST BE FILLED OUT IN THE SUBMITAL SET APPROVED BY DSA
 - IF NOTHING IS FILLED IN ON CWC7.1, POLYGON WILL ASSUME CUTOUPS ARE ONLY IN COLUMN A1 (SEE 'FRAMING PLAN' FOR REFERENCE)
- SELECT CLEAR HEIGHT (SEE 'ARCHITECTURAL VIEWS' SHEET FOR REFERENCE)
- IF NOTHING IS SELECTED, POLYGON WILL ASSUME THE DEFAULT FOR EACH DESIGN OPTION

STEP 3: IDENTIFY THE Ss & \$1 ACCELERATION (g) FOR YOUR PROJECT AND GEOTECHNICAL INFORMATION

- \$s & \$1 VALUE DETERMINES THE REQUIRED SEISMIC DESIGN FORCES
- \$s & \$1 VALUE DEPENDS ON PROJECTS GEOGRAPHICAL LOCATION
- FIND \$s & \$1 VALUES ARE SUGGESTED BECAUSE THEY ARE THE MOST ECONOMICAL
- FIND \$s & \$1 VALUES FOR YOUR PROJECT USING <https://asce7/hazardtool.online/>
- THIS PC IS NOT APPROVED FOR \$s VALUES GREATER THAN 2.063 (CONTACT POLYGON FOR ADDITIONAL OPTIONS)

STEP 4: IDENTIFY THE SEISMIC REGION FOR YOUR PROJECT

- THE REGIONS ARE DEPENDANT ON THE \$s & \$1 VALUES DETERMINED IN STEP 3
- THE SEISMIC REGION DICTATES THE MAXIMUM DEAD LOAD PERMITTED (SEE TABLE TO THE LEFT)

STEP 5: IDENTIFY THE ROOF DEAD LOAD FOR YOUR PROJECT

- THE ROOF DECK DEAD LOAD WILL ALWAYS BE INCLUDED
- THE COLLATERAL LOAD REPRESENTS ADDITIONAL LOAD THAT CAN BE SUPPORTED BY THE FRAME
- TOTAL ROOF DEAD LOAD MUST BE LESS THAN OR EQUAL TO THE MAX DEAD LOAD SHOWN IN STEP 4
- CUT SHEETS OF ANY BOARDS, BOXES AND EQUIPMENT TO BE MOUNTED ON THE STRUCTURE, INCLUDING WEIGHTS AND DIMENSIONS ARE REQUIRED

STEP 6: IDENTIFY THE LOAD SCENARIO

- REFERENCE THE STEP 4 COLOR AND SELECT THE APPLICABLE LOAD SCENARIO
- LOAD SCENARIOS HAVE NO IMPACT ON FRAME DESIGN OR COST, BUT DO AFFECT FOUNDATION SIZE

STEP 7: IDENTIFY PC STRUCTURE

- ROOF WIDTHS UP TO 10' WIDE USE THE "CWC 10"
- ROOF WIDTHS UP TO 15' WIDE USE THE "CWC 15"
- THE 10' AND 15' WIDTHS ARE SUGGESTED BECAUSE THEY ARE THE MOST ECONOMICAL
- MAXIMUM WIDTH IS 15'; (SEE 'ARCHITECTURAL VIEWS' SHEET FOR REFERENCE)

STEP 8: IDENTIFY SITE SPECIFIC ROOF WIDTH AND LENGTH

- DO NOT EXCEED THE TOTAL ROOF AREA FROM STEP 1 (ROOF WIDTH MULTIPLIED BY ROOF LENGTH)

STEP 9: FOUNDATION TYPE

- SELECT A FOUNDATION BASED THE DESIRED FOUNDATION TYPE
- SELECT EITHER SPREAD PAD OR DRILLED PIER FOUNDATION PRIOR TO APPROVAL
- FOUNDATION TYPE IMPACTS CONSTRUCTION TIMING, SEQUENCE, COST, ETC.
- REVIEW OF SITE-SPECIFIC SOILS REPORT TO EVALUATE APPLICABILITY OF FOUNDATION OPTIONS AVAILABLE

STEP 10: FOUNDATION SUMMARY

- USE THE SELECTIONS FROM STEP 6 AND STEP 9 TO SELECT THE APPROPRIATE FOUNDATION

STEP 11: SELECT APPLICABLE SHEET INDEX FOR YOUR PROJECT

- IDENTIFY THE APPLICABLE SHEET INDEX
- INCLUDE APPLICABLE SHEETS WITH YOUR DSA SUBMITAL
- EXCLUDE "MISC DESIGN OPTIONS" SHEET FOR PROJECTS WITHOUT ELECTRICAL CUTOUPS OR GUTTERS
- EXCLUDE "ELECTRICAL CUTOUPS" SHEET FOR PROJECTS WITHOUT ELECTRICAL CUTOUPS

STEP 12: MULTIPLE STRUCTURES WITH THE SAME PC #

- FILL IN ROOF LENGTH AND WIDTH OF STRUCTURES AS WELL AS QUANTITY
- ONLY ON THE POLYGON DRAWINGS, POLYGON WILL ASSUME ALL DESIGN CRITERIA FOR EACH STRUCTURE IS THE SAME
- CONTACT POLYGON FOR FURTHER INFORMATION

STEP 13: COLUMN BASE PROTECTION - SPREAD PAD FOUNDATION SELECTION ONLY

- SELECT THE METHOD OF COLUMN INSTALLATION ON APPLICABLE FOUNDATION PLAN SHEET, DETAIL 2, NOTE 3 SPREAD PAD FOUNDATION IS SELECTED

SHEET INDEX			
1 CWC1.0	ORDER FORM	11 CWC5.0	ARCHITECTURAL VIEWS - CWC 10
2 CWC1.1	NOTES AND SPECIAL INSPECTIONS	12 CWC5.1	ARCHITECTURAL VIEWS - CWC 15
3 CWC2.0	FOUNDATION PLAN SPREAD PAD - CWC 10	13 CWC6.0	ROOF CONNECTION DETAILS
4 CWC2.1	FOUNDATION PLAN DRILLED PIER - CWC 10	14 CWC6.1	ROOF CONNECTION DETAILS
5 CWC2.2	FOUNDATION PLAN SPREAD PAD - CWC 15	15 CWC7.0	MISC DESIGN OPTIONS
6 CWC2.3	FOUNDATION PLAN DRILLED PIER - CWC 15	16 CWC7.1	ELECTRICAL CUTOUPS
7 CWC3.0	FRAMING PLAN - CWC 10		
8 CWC3.1	FRAMING PLAN - CWC 15		
9 CWC4.0	FRAME CONNECTION DETAILS - CWC 10		
10 CWC4.1	FRAME CONNECTION DETAILS - CWC 15		
TOTAL SHEETS = 16			

ABBREVIATIONS:

ACI	AMERICAN CONCRETE INSTITUTE	MR	MULTI-RIB ROOF PANEL (MCELROY)
AISC	AMERICAN INSTITUTE OF STEEL CONSTRUCTION	NTS	NOT TO SCALE
ASM	ASSEMBLY (INTERNAL REFERENCE)	NO	NUMBER
ASTM	AMERICAN SOCIETY FOR TESTING AND MATLS	OC	ON CENTER
AWS	AMERICAN WELDING SOCIETY	OSHA	OCCUPATIONAL HEALTH AND SAFETY ADM.
CBC	CALIFORNIA BUILDING CODE	PCF	POUNDS PER CUBIC FOOT
CJP	COMPLETE JOINT PENETRATION	PD	POLYGON DRAWING
CLR	CLEAR	PJ	PRETENSIONED JOINT
DEG	DEGREE	PLCS	PLACES
DIA	DIAMETER	PLT	PLATE
DIM	DIMENSION	PSF	POUNDS PER SQUARE FOOT
DSA	DIVISION OF THE STATE ARCHITECT	PSI	POUNDS PER SQUARE INCH
EQ	EQUAL	QTY	QUANTITY
FT	FEET	REF	REFERENCE
GA	GAGE	SQ	SQUARE
IN	INCHES	SS	STANDING SEAM ROOF PANEL (MCELROY)
KSI	KIPS PER SQUARE INCH	TYP	TYPICAL
MAX	MAXIMUM	UNO	UNLESS NOTED OTHERWISE
MIN	MINIMUM	USGS	U.S. GEOLOGICAL SURVEY

GENERAL:

- 1. GENERAL NOTES AND TYPICAL DETAILS SHALL APPLY TO ALL PARTS OF THE JOB EXCEPT WHERE THEY MAY CONFLICT WITH DETAILS AND NOTES ON OTHER SHEETS. WHERE CONDITIONS ARE NOT SPECIFICALLY INDICATED BUT ARE OF SIMILAR CHARACTER TO DETAILS SHOWN, SIMILAR DETAILS OF CONSTRUCTION SHALL BE USED SUBJECT TO REVIEW BY THE STRUCTURAL ENGINEER FOR THIS PROJECT.
2. WORK SHALL CONFORM TO THE REQUIREMENTS, AS AMENDED TO DATE, OF THE LATEST ADOPTED EDITION OF THE CBC, C.A.C. TITLE 24, AND ALL OTHER LOCAL, STATE AND FEDERAL REGULATIONS.
3. OMISSIONS OR CONFLICTS BETWEEN THE VARIOUS ELEMENTS OF THE WORKING DRAWINGS AND/OR SPECIFICATIONS SHALL BE BROUGHT TO THE ATTENTION OF THE STRUCTURAL ENGINEER FOR THIS PROJECT PRIOR TO PROCEEDING WITH ANY WORK INVOLVED.
4. THE CONTRACTOR SHALL BE RESPONSIBLE FOR COORDINATING THE WORK OF ALL TRADES AND SHALL CHECK ALL DIMENSIONS. ALL DISCREPANCIES SHALL BE CALLED TO THE ATTENTION OF THE STRUCTURAL ENGINEER FOR THIS PROJECT AND BE RESOLVED BEFORE PROCEEDING WITH THE WORK.
5. THESE CONSTRUCTION DRAWINGS AND SPECIFICATIONS REPRESENT THE FINISHED STRUCTURE AND DO NOT INDICATE THE METHOD OF CONSTRUCTION. THE CONTRACTOR SHALL SUPERVISE AND DIRECT THE WORK AND SHALL BE SOLELY RESPONSIBLE FOR CONSTRUCTION MEANS, METHODS, TECHNIQUES, SEQUENCES AND PROCEDURES, INCLUDING, BUT NOT LIMITED TO, BRACING, TEMPORARY SUPPORTS, AND SHORING. OBSERVATION VISITS TO THE SITE BY FIELD REPRESENTATIVES OF THE ARCHITECT/ENGINEER SHALL NOT INCLUDE INSPECTIONS OF THE PROTECTIVE MEASURES OR THE CONSTRUCTION PROCEDURES. ANY SUPPORT SERVICES PERFORMED BY THE ARCHITECT/ENGINEER DURING THE CONSTRUCTION SHALL BE DISTINGUISHED FROM CONSTRUCTION AND DETAILED INSPECTION SERVICES WHICH ARE FURNISHED BY OTHERS. THESE SUPPORT SERVICES PERFORMED BY THE ARCHITECT/ENGINEER, WHETHER OF MATERIAL OR WORK, ARE FOR THE PURPOSE OF ASSISTING IN QUALITY CONTROL AND IN ACHIEVING CONFORMANCE WITH CONTRACT DOCUMENTS, BUT DO NOT GUARANTEE CONSTRUCTION.
6. ASTM DESIGNATIONS AND ALL STANDARDS REFER TO THE LATEST AMENDMENTS.
7. CONFORM TO APPLICABLE CAL/OSHA CONSTRUCTION SAFETY REGULATIONS FOR ALL WORK PERFORMED DURING CONSTRUCTION. JOB SITE SAFETY IS STRICTLY THE RESPONSIBILITY OF THE CONTRACTOR AND NOT THE ARCHITECT/ENGINEER OR OWNER.
8. THE ENGINEER AND THEIR CONSULTANTS SHALL HAVE NO RESPONSIBILITY FOR THE DISCOVERY, HANDLING, REMOVAL OR DISPOSAL OF HAZARDOUS MATERIALS AT THE PROJECT SITE, INCLUDING BUT NOT LIMITED TO, ASBESTOS, ASBESTOS PRODUCTS, POLYCHLORINATED BIPHENYL (PCB) OR OTHER TOXIC SUBSTANCES.
9. SHOULD ANY CONDITIONS DEVELOP NOT COVERED BY THE CONTRACT DOCUMENTS, OR IF A CHANGE IN THE SCOPE OF WORK IS PROPOSED, A CONSTRUCTION CHANGE DOCUMENT DETAILING AND SPECIFYING THE REQUIRED CHANGE(S) SHALL BE SUBMITTED TO AND APPROVED BY DSA BEFORE PROCEEDING WITH THE WORK.
10. THE SCHOOL DISTRICT'S INSPECTOR OF RECORD SHALL INSPECT AND APPROVE THE ERECTED FRAME PRIOR TO ROOF INSTALLATION.
11. SEE REQUIREMENTS FOR LOCATION IN ANY FIRE HAZARD SEVERITY ZONE FOR WILDLAND URBAN INTERFACE AREAS (WUI) AS SPECIFIED IN THE APPLICABLE VERSION OF THE CALIFORNIA BUILDING CODE. PROVIDE PROTECTION AND DETAILS OF ALL AREAS COMPLYING WITH THE WUI REQUIREMENTS.
12. LOCATING THIS STRUCTURE CLOSER THAN 20 FEET TO OTHER STRUCTURES MAY AFFECT THE ALLOWABLE AREA FOR THE EXISTING CONSTRUCTION PER THE APPLICABLE VERSION OF THE CALIFORNIA BUILDING CODE.
13. VIEWS AND DETAILS ARE NOT DRAWN TO SCALE (UNLESS NOTED OTHERWISE). DO NOT SCALE THESE DRAWINGS.
14. OTHER SITE SPECIFIC ITEMS MAY BE REQUIRED.
15. WHEN A SITE-SPECIFIC PROJECT IS LOCATED IN A FLOOD ZONE OTHER THAN ZONE X, A LETTER STAMPED AND SIGNED FROM A SOILS ENGINEER IS NEEDED TO VALIDATE THE ALLOWABLE SOIL VALUES SPECIFIED IN THE PC ARE STILL APPLICABLE.

STRUCTURAL AND MISCELLANEOUS STEEL:

- 1. ALL STRUCTURAL STEEL SHALL BE DETAILED, FABRICATED AND ERECTED IN ACCORDANCE WITH THE AMERICAN INSTITUTE OF STEEL CONSTRUCTION (AISC) 360-16 AND 303-16 REFERENCED BY THE 2022 EDITION OF THE CALIFORNIA BUILDING CODE.
2. PIPE SECTIONS SHALL CONFORM TO ASTM A53, Fy = 35 ksi, GRADE B UNLESS NOTED OTHERWISE.
3. STRUCTURAL TUBING (HSS SHAPES) SHALL CONFORM TO ASTM A500, GRADE B (OR HIGHER), Fy = 46 KSI.
4. IF MATERIAL AVAILABILITY IS LIMITED, MEMBER THICKNESSES CAN BE INCREASED BEYOND WHAT IS SHOWN IN THESE DRAWINGS (MAXIMUM INCREASE OF 1/8").
5. ALL CHANNELS, ANGLES, PLATES AND MISC. STEEL SHALL CONFORM TO ASTM A36, Fy = 36 KSI.
6. ALL COLD FORM STEEL SHALL CONFORM TO ASTM A653, CS = TYPE B, Fy = 50 KSI.
7. STRUCTURAL STEEL AND DECK SHALL BE IDENTIFIED FOR CONFORMITY PER CBC 2202A.1.
8. ROOF DECK SHALL HAVE KYNAR 5000 METAL COATING.
9. ROOF DECK SHALL CONFORM TO ASTM A792, Fy = 50 KSI.
10. MR ROOF SCREWS MEET ASTM A510 WITH A HEAD DIMENSION OF 0.31" (FLAT-TO-FLAT) AND INTEGRAL WASHER DIMENSION OF 0.58" (OUTSIDE DIAMETER).
11. SS ROOF SCREWS MEET ASTM A510 WITH A HEAD DIMENSION OF 0.437" (OUTSIDE DIAMETER).

WELDING:

- 1. ALL WELDING SHALL COMPLY WITH AWS D1.1 SPECIFICATIONS AND SHALL BE DONE BY AWS QUALIFIED WELDERS CERTIFIED FOR THE TYPE OF WELDING TO BE PERFORMED.
2. ALL WELDING SHALL BE DONE BY GAS METAL ARC PROCESS WITH E70XX ELECTRODES. FLUX CORE ARC WELD SHALL CONFORM TO CHARPY NOTCH TOUGHNESS RATING OF 20 ft-lb @ (0° F).
3. ALL WELDING SHALL BE DONE IN THE SHOP WITH REQUIRED INSPECTION, PRE-APPROVED BY DSA, TO ENSURE PROPER MATERIAL ID AND WELDING.
4. WELD FILLER METAL MANUFACTURER SHALL PROVIDE WRITTEN CERTIFICATION OF COMPLIANCE WITH CODE AND SPECIFICATIONS.

BOLTING:

- 1. ALL BOLTS SHOWN ON THESE DRAWINGS ARE ASTM F3125 (A325 TYPE 1) HIGH STRENGTH BOLTS (UNO) AND SHALL BE HOT DIPPED GALVANIZED PER ASTM F2329.
2. HIGH STRENGTH BOLTS SHALL BE SAMPLED AND TESTED IN COMPLIANCE WITH CBC 2213A.1.
3. BEFORE ERECTING THE FRAME, VERIFY ALL BOLTS AND NUTS ARE CLEAN OF DEBRIS AND BURRS - INCLUDING THE HARDWARE ALREADY FASTENED INSIDE THE MEMBERS. CHASING SOME OF THE BOLTS AND NUTS MAY BE REQUIRED.
4. ANCHOR BOLTS (HEAVY HEX HEAD, ASTM F1554, GRADE 5S) SHALL BE HOT DIPPED GALVANIZED PER ASTM F2329. ANCHOR BOLTS MAY BE HEADED OR THREADED WITH A NUT THAT IS PREVENTED FROM ROTATING.
5. HIGH STRENGTH NUTS SHALL CONFORM TO ASTM A563 AND SHALL BE GALVANIZED PER ASTM F2329.
6. HIGH STRENGTH WASHERS SHALL CONFORM TO ASTM F436 AND SHALL BE GALVANIZED PER ASTM F2329.
7. THE BOLTING INSTALLATION REQUIREMENTS OUTLINED BELOW ARE CRITICAL TO THE STRUCTURE'S DESIGN AND PERFORMANCE. THE INSTALLER IS REQUIRED TO COORDINATE THIS PHASE OF CONSTRUCTION WITH THE SPECIAL BOLTING INSPECTOR AND THE INSPECTOR OF RECORD PRIOR TO THE ERECTION OF THE FRAME. ALL BOLTS SHALL BE INSTALLED AND INSPECTED PER THE APPLICABLE VERSION OF AISC'S SPECIFICATION FOR STRUCTURAL JOINTS USING HIGH-STRENGTH BOLTS, CBC 1705A.2.1; AISC 341-16 J7; AISC 360-16 N5.6.
A. PRETENSIONED JOINTS (IDENTIFIED ON THE FRAME CONNECTION DETAILS WITH A "PJ REQUIRED") MUST BE INSTALLED AND INSPECTED TO MEET ONE OF FOLLOWING REQUIREMENTS:
1. TURN-OF-NUT PRETENSIONING
2. CALIBRATED WRENCH PRETENSIONING
3. DIRECT-TENSION-INDICATOR PRETENSIONING (CONTRACTOR RESPONSIBLE FOR PURCHASE OF REQUIRED WASHERS)
B. ALL OTHER JOINTS MUST BE INSTALLED AND INSPECTED TO MEET THE REQUIREMENTS OF SNUG-TIGHTENED JOINTS. NOTE TO INSTALLER AND INSPECTOR(S): THE SNUG-TIGHT CONDITION EXISTS, IN PART, WHEN ALL THE BOLTS IN THE JOINT HAVE BEEN TIGHTENED SUFFICIENTLY TO PREVENT THE REMOVAL OF THE NUTS WITHOUT THE USE OF A WRENCH.

THE CONTRACTOR, SPECIAL BOLTING INSPECTOR AND THE INSPECTOR OF RECORD MUST ALL AGREE ON WHICH APPROACH WILL BE USED TO PRETENSION THE BOLTS. THE CONTRACTOR IS RESPONSIBLE FOR DOCUMENTING THE APPROACH AGREED TO BY ALL PARTIES LISTED ABOVE.

FOUNDATIONS:

- 1. ALLOWABLE SOIL PRESSURES ASSUME CLASS 5 SOIL CLASSIFICATION PER 2022 CBC TABLE 1806A.2
2. FILL AND BACKFILL SHALL BE COMPACTED TO 95% OF MAX. DENSITY IN ACCORDANCE WITH ASTM TEST METHOD D1557. FLOODING NOT PERMITTED.
3. THE CONTRACTOR SHALL BE RESPONSIBLE FOR ALL SHORING, ETC. NECESSARY TO SUPPORT CUT AND/OR FILL BANKS DURING EXCAVATION, AND FORMING AND PLACEMENT OF CONCRETE.
4. STRUCTURES SHALL BE SETBACK FROM ADJACENT SLOPES TO PROVIDE FIRM MATERIAL FOR EMBEDMENT AND FOR PROTECTION FROM SLOPE DRAINAGE, EROSION, AND SHALLOW FAILURES.
A. BOTTOM OF ASCENDING SLOPE: THE SMALLER OF HALF THE HEIGHT OF THE SLOPE AND 15FT MEASURED FROM THE FACE OF THE STRUCTURE TO THE TOE OF THE SLOPE
B. TOP OF DECENDING SLOPE: THE SMALLER OF A THIRD OF THE HEIGHT OF THE SLOPE AND 40 FT MEASURED FROM THE FACE OF THE FOOTING TO THE TOP OF THE SLOPE
ALTERNATE SETBACKS ARE PERMITTED, SUBJECT FOR APPROVAL. A GEOTECHNICAL INVESTIGATION MAY BE REQUIRED.
5. STRUCTURES PLACED ON LIQUIFIABLE SOILS OR SITE CLASS F MAY NOT BE SUBMITTED FOR AN OVER THE COUNTER REVIEW.

CONCRETE:

- 1. MIX DESIGN REQUIREMENTS: (NORMAL WEIGHT CONCRETE)
Table with 5 columns: MINIMUM STRENGTH Fc [28 DAYS], EXPOSURE CATEGORY, MAXIMUM W/C RATIO, SLUMP (± 1"), UNIT WEIGHT (NORMAL WEIGHT)
5000 PSI, F3, S3, W2, C2, 0.4, 4", 150 PCF
2. CHANGES TO THE MIX DESIGN MUST BE APPROVED BY THE ENGINEER OR ARCHITECT OF RECORD AND DSA
3. AGGREGATES SHALL CONFORM TO ASTM C33. MAX AGGREGATE SIZE = 1".
4. CEMENT SHALL CONFORM TO ASTM C150 (TYPE V) WITH A MAXIMUM EXPANSION OF 0.040%, FOR SULFATE RESISTANCE.
5. ADMIXTURES CONTAINING CALCIUM CHLORIDE ARE PROHIBITED.
6. CONCRETE EXPOSED TO FREEZING-AND-THAWING CYCLES SHALL BE AIR ENTRAINED PER ACI 318-19 SECTION 19.3.3.
7. CONCRETE SHALL BE MAINTAINED IN A MOIST CONDITION FOR A MINIMUM OF FIVE DAYS AFTER PLACEMENT. ALTERNATE METHODS WILL BE APPROVED IF SATISFACTORY PERFORMANCE CAN BE ASSURED.
8. CONCRETE SHALL NOT FREE FALL MORE THAN FIVE FEET.
9. CONCRETE SHALL BE PROPORTIONED PER ACI 318-19 26.4.
10. CONCRETE SHALL BE TESTED PER CBC 1910A.1, 1705A.3, AND ACI 318-19 26.13. BATCH PLANT INSPECTION NOT REQUIRED. CONTRACTOR SHALL IMPLEMENT WEIGHTMASTER AND BATCH TICKET REQUIREMENTS OF CBC 1705A.3.3.1.

REINFORCING STEEL:

- 1. REINFORCING STEEL SHALL BE DEFORMED STEEL CONFORMING TO THE REQUIREMENTS OF ASTM A615. (DEFORMATIONS SHALL BE IN ACCORDANCE WITH ASTM A305) AS FOLLOWS:
GR 60: (#4 BARS AND LARGER)
GR 40: (#3 BARS)
2. DETAILING, FABRICATION, AND ERECTION OF REINFORCING BARS SHALL CONFORM TO THE ACI "MANUAL OF STANDARD PRACTICE FOR DETAILING REINFORCING CONCRETE STRUCTURES."
3. MIN. COVER FOR CAST-IN-PLACE CONCRETE SHALL BE AS FOLLOWS:
A. CAST AGAINST EARTH..... 3"
B. CAST AGAINST FORM BELOW GRADE..... 2"
C. FORMED SLABS (#1 BAR & SMALLER)..... 3/4"
D. SLABS ON GRADE (FROM TOP OF SLAB)..... 1"
E. COLUMNS AND BEAMS (MAIN BARS)..... 1"
F. WALLS EXPOSED TO WEATHER (#6-#18 BARS)..... 2"
G. (#5 & SMALLER)..... 1 1/2"
H. NOT EXPOSED TO WEATHER (#11 & SMALLER)..... 3/4"
4. BARS SHALL BE CLEAN OF RUST, GREASE OR OTHER MATERIAL LIKELY TO IMPAIR BOND. BENDS SHALL BE MADE COLD.
5. FOR #6 BARS AND SMALLER, REINFORCING SHALL BE LAP SPICED 45 BAR DIA MINIMUM IN CONCRETE. FOR #7 BARS AND LARGER, REINFORCING SHALL BE LAP SPICED 55 BAR DIAMETERS MINIMUM IN CONCRETE. ALL LAP SPICES MUST COMPLY WITH ACI 318-19.
6. PRIOR TO PLACING OF CONCRETE, REINFORCING STEEL AND EMBEDDED ITEMS SHALL BE WELL SECURED IN POSITION.
7. WELDING OF REINFORCING IS NOT ALLOWED
8. REINFORCING STEEL SHALL BE SAMPLED AND TESTED PER CBC 1910A.2.

POWDER COATED AND EPOXY PRIMED FINISH:

- 1. ENTIRE POWDER COATING PROCESS COMPLETED IN SAME FACILITY AS STEEL FABRICATION.
2. ALL CARBON STEEL MEMBERS (COLUMNS, BEAMS, PLATES, ETC.) PAINTED WITH PRIME COAT PER THE "AISC CODE OF STANDARD PRACTICE" AND THE "AISC SPECIFICATION SECTION M3" (UNLESS NOTED OTHERWISE).
3. PARTS PRETREATED IN A 3 STAGE IRON PHOSPHATE WASHER (OR EQUAL).
4. EPOXY PRIMER POWDER COAT APPLIED TO PARTS FOR SUPERIOR CORROSION PROTECTION.
5. TOP POWDER COAT OF SUPER DURABLE TGIC (COLOR SELECTED FROM MANUFACTURER'S STANDARD OPTIONS OR CUSTOM COLOR).
6. SAMPLE PRODUCTION PARTS TESTED TO MEET THE FOLLOWING CRITERIA:
A. SALT SPRAY RESISTANCE PER ASTM B 117/ ASTM D 1654
1. 1000 HOURS WITH NO CREEP FROM SCRIBE LINE AND RATING OF 10
B. HUMIDITY RESISTANCE PER ASTM D2247-02
1. 5000 HOURS WITH NO LOSS OF ADHESION OR BLISTERING
C. COLOR/FY RESISTANCE PER ASTM G154-04
1. 2000 HOURS EXPOSURE ALTERNATE CYCLES WITH NO CHALKING, 75% COLOR RETENTION, AND COLOR VARIATION MAXIMUM 3.0 E VARIATION CIE FORMULA (BEFORE AND AFTER 2000 HOURS EXPOSURE)

CONSTRUCTION NOTES

- 1. A DSA-CERTIFIED CLASS 2 INSPECTOR IS REQUIRED FOR THIS PROJECT.
2. CHANGES TO THE APPROVED DRAWINGS AND SPECIFICATIONS SHALL BE MADE BY ADDENDA OR CONSTRUCTION CHANGE DOCUMENT (CCD) APPROVED BY DSA, AS REQUIRED BY SECTION 4-338, PART 1, TITLE 24 CCR, AND DSA IR A-6.
3. A "DSA-CERTIFIED" PROJECT INSPECTOR EMPLOYED BY THE OWNER (E.G. DISTRICT, ETC.) AND APPROVED BY DSA SHALL PROVIDE CONTINUOUS INSPECTION OF WORK. THE DUTIES OF THE INSPECTOR ARE DEFINED IN SECTION 4-342, PART 1, TITLE 24, CCR.
4. A DSA ACCEPTED TESTING LABORATORY DIRECTLY EMPLOYED BY THE OWNER (E.G. DISTRICT, ETC.) SHALL CONDUCT ALL THE REQUIRED TEST AND INSPECTIONS FOR THE PROJECT.

NOTICE OF DISCLAIMER FOR STRUCTURAL ENGINEER RESPONSIBILITY

- 1. FOR THE SITE-SPECIFIC PROJECT, NEITHER POLYGON OR GHD ARE THE DESIGN PROFESSIONAL IN GENERAL RESPONSIBLE CHARGE.
2. FOR THE SITE-SPECIFIC PROJECT, GHD AND POLYGON'S RESPONSIBILITY IS LIMITED TO THE PREPARATION OF THE PLANS AND SPECIFICATIONS FOR THE STRUCTURES OF THIS PC ONLY.
3. STRUCTURAL OBSERVATION OF CONSTRUCTION IS SPECIFICALLY EXCLUDED FROM GHD AND POLYGON'S RESPONSIBILITY FOR THE SITE-SPECIFIC PROJECT.
4. ALL CONSTRUCTION ACTIVITIES RELATED TO STRUCTURAL ENGINEERING MAY BE DELEGATED TO A QUALIFIED ENGINEER BY THE DESIGN PROFESSIONAL IN GENERAL RESPONSIBLE CHARGE. THESE ACTIVITIES INCLUDE, BUT ARE NOT LIMITED TO, APPROVAL OF INSPECTOR QUALIFICATIONS, STRUCTURAL OBSERVATIONS OF CONSTRUCTION, REVIEW OF INSPECTIONS REPORTS, AND SIGNING OFF ON THE VERIFIED REPORT FOR COMPLETED WORK.
5. POLYGON WILL BE RESPONSIBLE FOR RESPONDING TO QUESTIONS PERTAINING TO THE PLANS AND SPECIFICATIONS FOR THE STRUCTURES OF THIS PC WHICH ARISE DURING PLAN REVIEW AND CONSTRUCTION.

SPECIAL INSPECTION NOTES:

- 1. THE PROJECT INSPECTOR AND TESTING AGENCY SHALL BE SELECTED BY THE SCHOOL DISTRICT AND APPROVED BY DSA AND THE ARCHITECT OF RECORD.
2. COSTS OF THE PROJECT INSPECTOR AND THE TESTING AGENCY SHALL BE BORN BY THE SCHOOL DISTRICT.
3. THE PROJECT INSPECTOR, AND ENTIRE CONSTRUCTION OVERSIGHT PROCESS, SHALL COMPLY WITH DSA PR 13-01.
4. ON APPROVED PC DRAWINGS, THE STATEMENT OF STRUCTURAL TESTS AND SPECIAL INSPECTIONS (FORM DSA-103) BELOW IS ONLY AN EXAMPLE ON APPROVED PC DRAWINGS. THE EXAMPLE FORM DSA-103 MUST BE CROSSED OUT BEFORE THE PC DRAWINGS CAN BE APPROVED AS PART OF A SITE-SPECIFIC (OR STOCKPILE) PROJECT SO THEY WILL NOT CONFLICT WITH THE OFFICIAL FORM DSA-103 FOR THE PROJECT.

DSA 103-22: LISTING OF STRUCTURAL TESTS & SPECIAL INSPECTIONS, 2022 CBC

Table with 3 columns: Application Number, School Name, School District, DSA File Number, Increment Number, Date Submitted.

IMPORTANT: This form is only a summary list of structural tests and some of the special inspections required for the project. Generally, the structural tests and special inspections noted on this form are those that will be performed by the Geotechnical Engineer of Record, Laboratory of Record, or Special Inspector. The actual complete test and inspection program must be performed as detailed on the DSA approved documents. The appendix at the bottom of this form identifies work NOT subject to DSA requirements for special inspection or structural testing. The project inspector is responsible for providing inspection of all facets of construction, including but not limited to, special inspections not listed on this form such as structural wood framing, high-load wood diaphragms, cold-formed steel framing, anchorage of non-structural components, etc., per Title 24, Part 2, Chapter 17A (2022 CBC). **NOTE: Undefined section and table references found in this document are from the CBC, or California Building Code.

KEY TO COLUMNS

Table with 2 columns: 1. TYPE, 2. PERFORMED BY. Includes definitions for Continuous, Periodic, Test, GE, LOR, and PI.

Table with 4 columns: S1. GENERAL, Type, Performed By, Code References and Notes. Includes S1.1 General, Test or Special Inspection, and Test or Special Inspection sub-table.

Table with 4 columns: S2. SOIL COMPACTION AND FILL, Type, Performed By, Code References and Notes. Includes S2.1 General and Test or Special Inspection sub-table.

Table with 4 columns: S4. CAST-IN-PLACE DEEP FOUNDATIONS (PIERS), Type, Performed By, Code References and Notes. Includes S4.1 General and Test or Special Inspection sub-table.

Table with 4 columns: C1. CAST-IN-PLACE CONCRETE, Type, Performed By, Code References and Notes. Includes C1.1 General and Test or Special Inspection sub-table.

Table with 4 columns: S/A1. STRUCTURAL STEEL, COLD-FORMED STEEL AND ALUMINUM USED FOR STRUCTURAL PURPOSES, Type, Performed By, Code References and Notes. Includes S/A1.1 General and Test or Special Inspection sub-table.

Table with 4 columns: S/A2. HIGH-STRENGTH BOLTS, Type, Performed By, Code References and Notes. Includes S/A2.1 General and Test or Special Inspection sub-table.

Table with 4 columns: S/A3. WELDING, Type, Performed By, Code References and Notes. Includes S/A3.1 General and Test or Special Inspection sub-table.

Table with 4 columns: S/A4. SHOP WELDING (IN ADDITION TO SECTION S/A3), Type, Performed By, Code References and Notes. Includes S/A4.1 General and Test or Special Inspection sub-table.

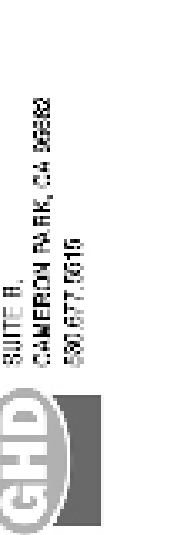
Table with 4 columns: S/A5. ANCHOR BOLTS AND ANCHOR RODS, Type, Performed By, Code References and Notes. Includes S/A5.1 General and Test or Special Inspection sub-table.

Name of Architect or Engineer in general responsible charge:
Name of Structural Engineer (When structural design has been delegated):
Signature of Architect or Structural Engineer:
Date:

Note: To facilitate DSA electronic mark-ups and identification stamp application, DSA recommends against using secured electronic or digital signatures.
DSA STAMP

DSA 103-22: LIST OF REQUIRED VERIFIED REPORTS, CBC 2022

- 1. Soils Testing and Inspection: Geotechnical Verified Report Form DSA 293
2. Structural Testing and Inspection: Laboratory Verified Report Form DSA 291
3. Shop Welding Inspection: Laboratory Verified Report Form DSA 291, or, for independently contracting SI, Special Inspection Verified Report Form DSA 292
4. High-Strength Bolt Installation Inspection: Laboratory Verified Report Form DSA 291, or, for independently contracting SI, Special Inspection Verified Report Form DSA 292



IDENTIFICATION STAMP
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REVIEWED FOR
SS [x] FLS [x] ACS [x] CG [x]
DATE: 7/14/2023

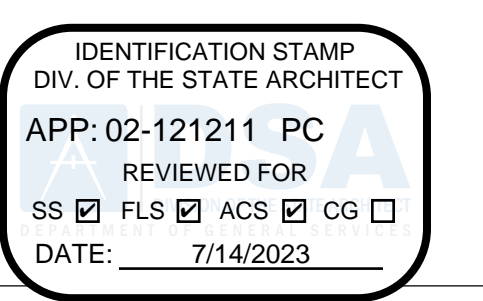
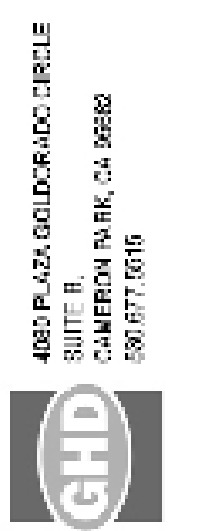
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DOCUMENT
CODE: 2022 CBC
A SEPARATE PROJECT
APPLICATION FOR
CONSTRUCTION IS REQUIRED.

NOTES AND SPECIAL
INSPECTIONS

WALKAWAY COVER - CWC

FOUNDATION PLAN NOTES:

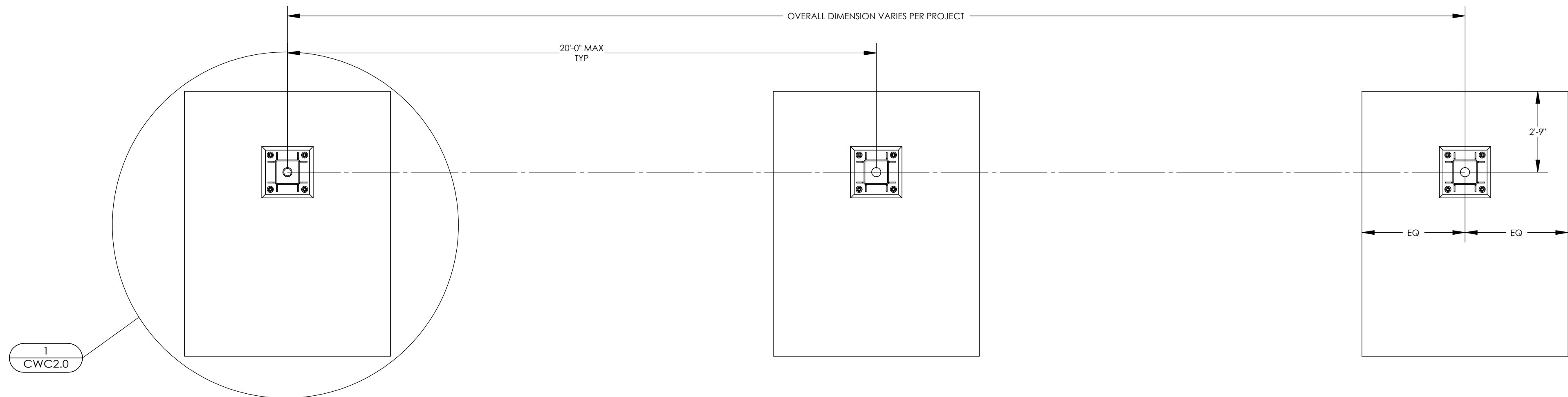
1. TOP OF ALL FOUNDATIONS MUST BE CONSTRUCTED AT ONE COMMON ELEVATION (COORDINATE WITH SITE PLANS - NOT BY POLYGON)
2. ALL FOUNDATIONS MUST BE CENTERED UNDER COLUMNS (UNO).
3. SEE SHEET CWC1.1 FOR CONCRETE REQUIREMENTS.
4. PRIOR TO FORMING AND CASTING FOUNDATIONS, REVIEW FOUNDATION PLAN FOR REQUIRED ORIENTATION.
5. FOUNDATION MATERIAL AND INSTALLATION NOT BY POLYGON.
6. VIBRATE CONCRETE FULL DEPTH OF FOUNDATION.



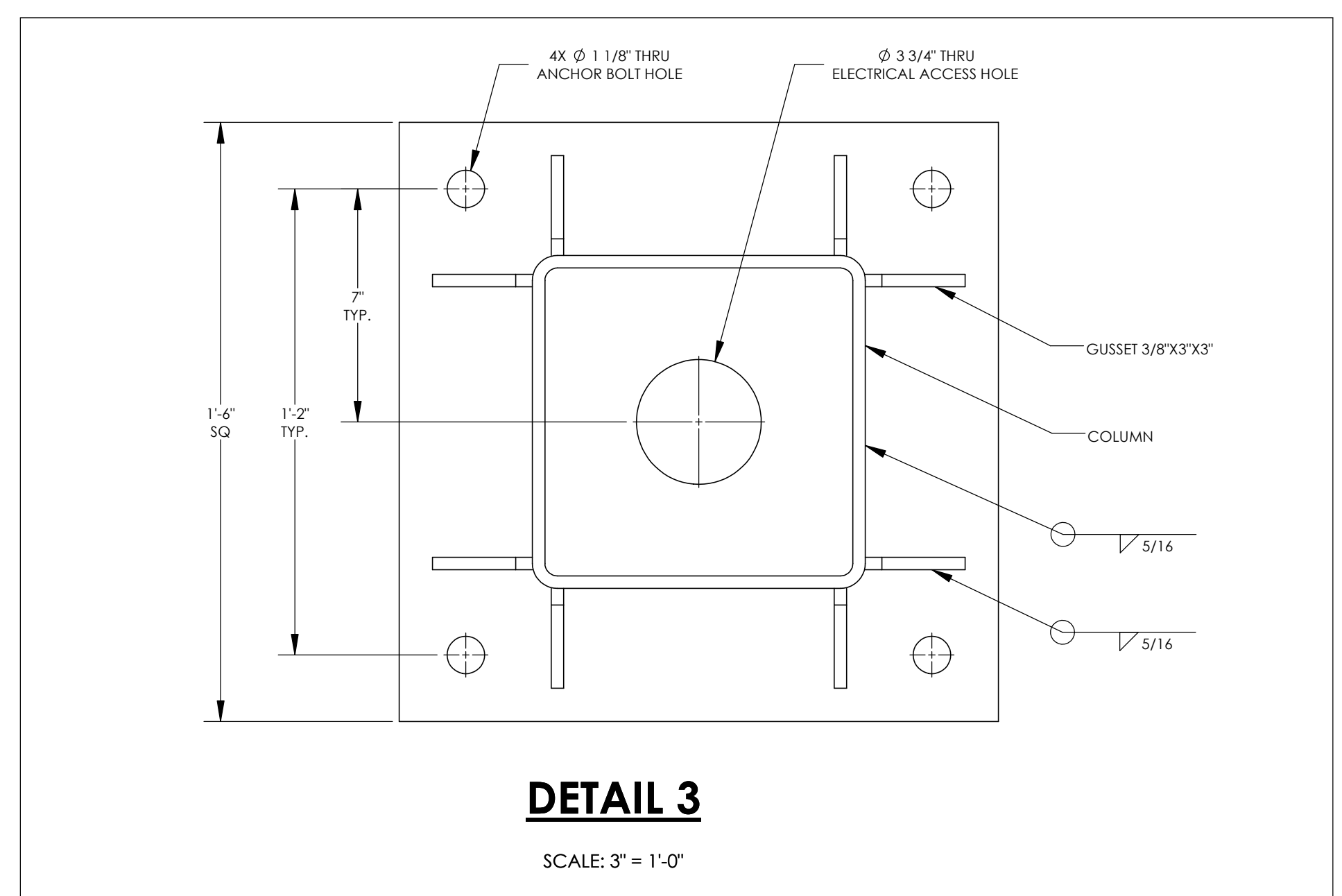
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 A SEPARATE PROJECT APPLICATION FOR CONSTRUCTION IS REQUIRED.

FOUNDATION PLAN SPREAD PAD
 WALKWAY COVER - CWC 10

CWC2.0



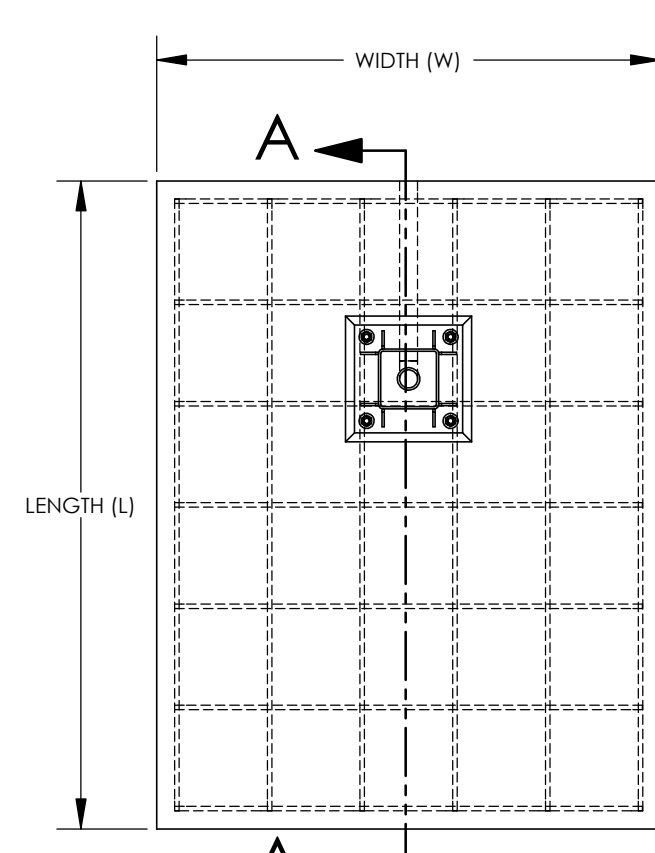
FOUNDATION PLAN (SPREAD PAD)
 SCALE: 3/8" = 1'-0"



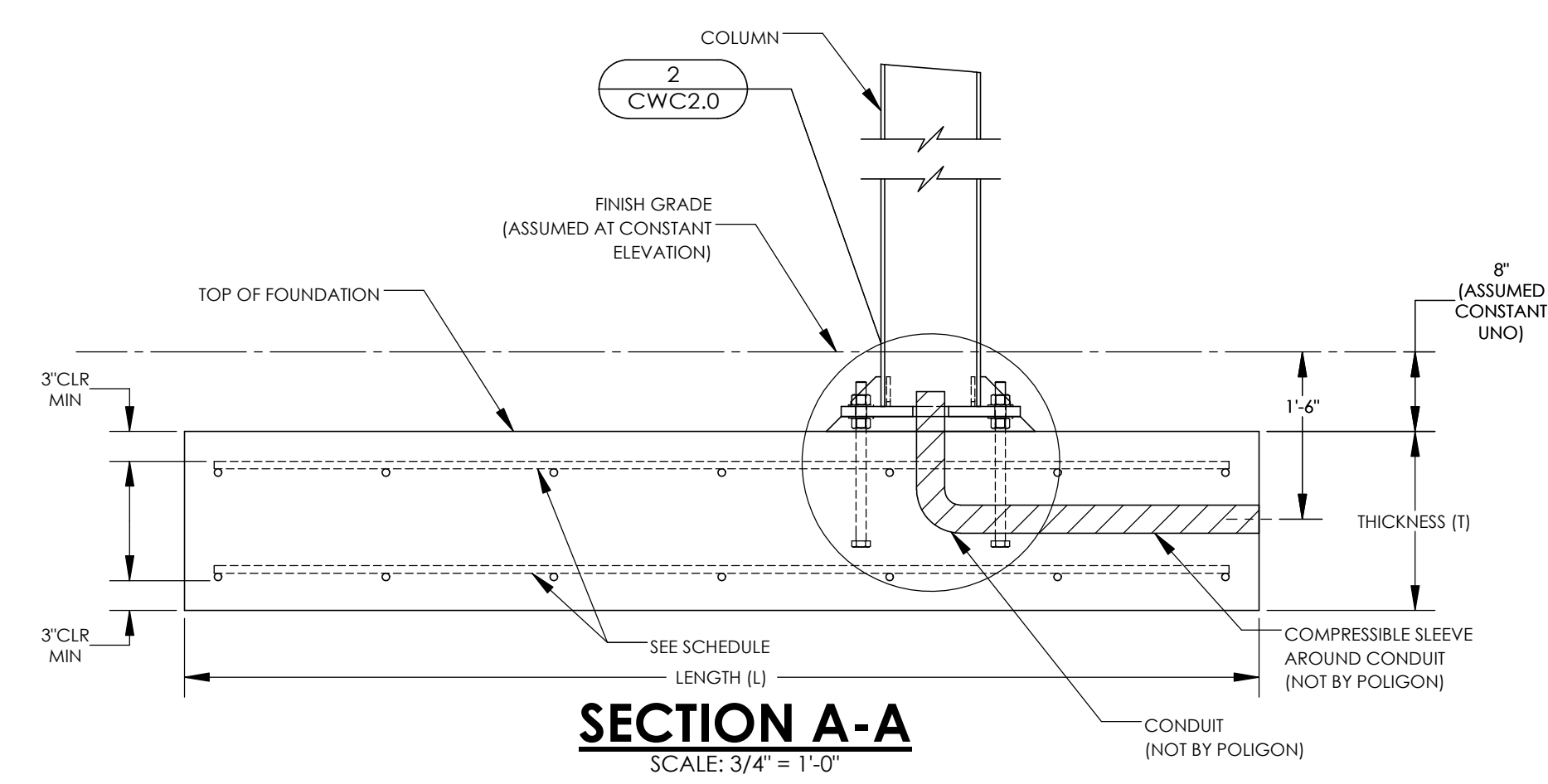
DETAIL 3
 SCALE: 3" = 1'-0"

COLUMN BASEPLATE

3



DETAIL 1
 SCALE: 3/8" = 1'-0"



SECTION A-A
 SCALE: 3/4" = 1'-0"

SPREAD PAD FOUNDATION

1

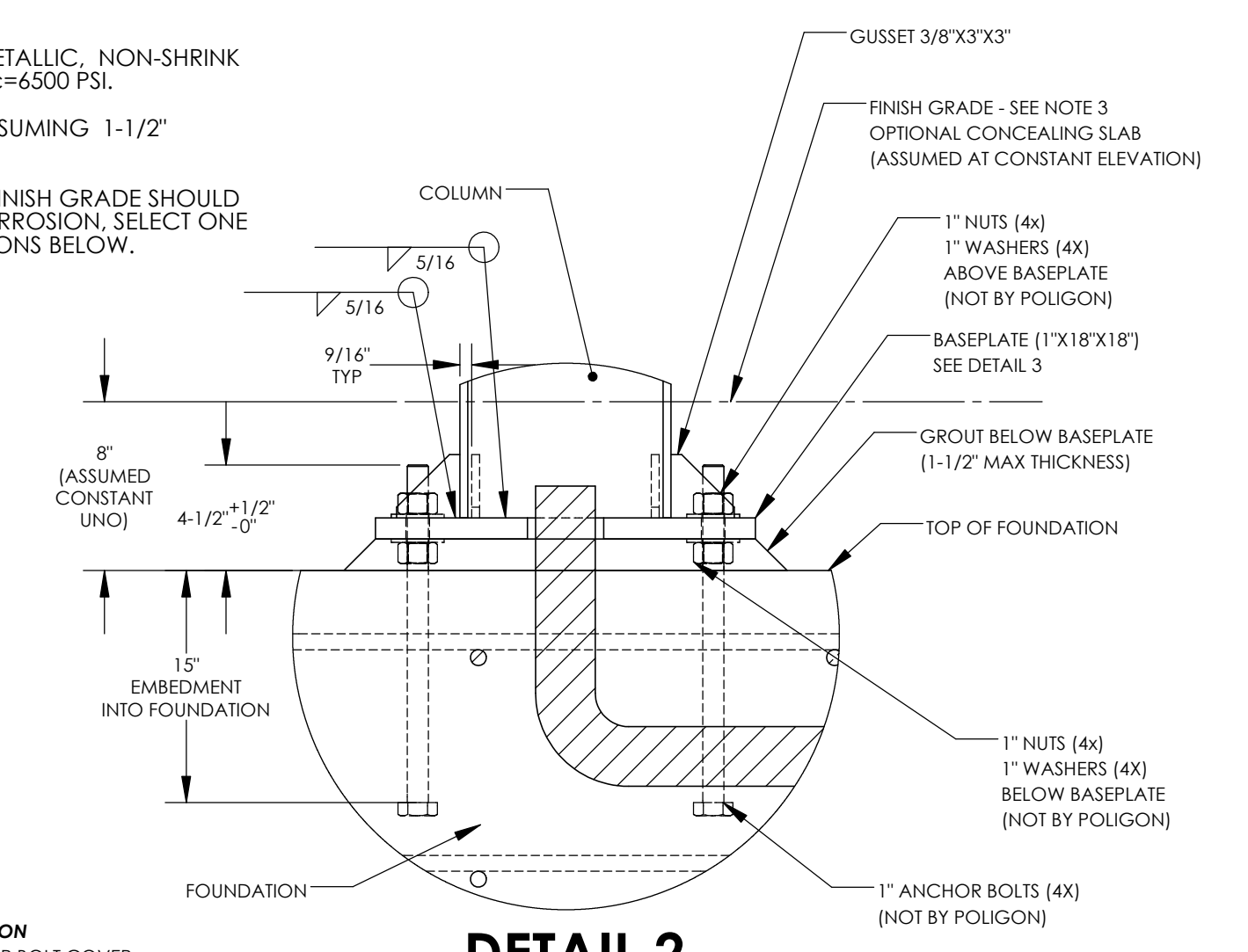
FOUNDATION REQUIREMENTS VARY PER PROJECT
 SEE SHEET CWC1.0 FOR REQUIRED LOAD SCENARIO AND FOUNDATION TYPE (STEP 9 OF 'INSTRUCTIONS')
 ONLY REFERENCE COPY OF PC DRAWINGS SUBMITTED FOR THIS PROJECT

SPREAD PAD					
SIZE AND REINFORCING REQUIREMENTS					
LOAD SCENARIO	WIDTH (W)	LENGTH (L)	THICKNESS (T)	HORIZONTAL REINFORCING ¹	
				QTY	SIZE
1	7'-6"	10'-0"	1'-6"	9	#6
2	8'-6"	11'-0"	1'-6"	10	#6

¹EQUALLY SPACED EACH WAY, TOP AND BOTTOM

NOTES:

1. GROUT SHALL BE NON-METALLIC, NON-SHRINK GROUT WITH MINIMUM Fc=6500 PSI.
2. COLUMNS FABRICATED ASSUMING 1-1/2" GROUT PAD.
3. COLUMN BASES BELOW FINISH GRADE SHOULD BE PROTECTED FROM CORROSION, SELECT ONE OF THE FOLLOWING OPTIONS BELOW.



DETAIL 2
 SCALE: 1-1/2" = 1'-0"

COLUMN BASE PROTECTION
 BELOW SURFACE COLUMN INSTALLATION
 [] CONCRETE SLAB - 3" MIN ANCHOR BOLT COVER
 [] MASTIC COATING - 1/4" THICK MIN COATING ON ALL STEEL SURFACES BELOW GRADE

COLUMN BASEPLATE AND ANCHOR BOLTS

2

FOUNDATION PLAN NOTES:

1. TOP OF ALL FOUNDATIONS MUST BE CONSTRUCTED AT ONE COMMON ELEVATION (COORDINATE WITH SITE PLANS - NOT BY POLYGON)
2. ALL FOUNDATIONS MUST BE CENTERED UNDER COLUMNS (UNO).
3. SEE SHEET CWC1.1 FOR CONCRETE REQUIREMENTS.
4. PRIOR TO FORMING AND CASTING FOUNDATIONS, REVIEW FOUNDATION PLAN FOR REQUIRED ORIENTATION.
5. FOUNDATION MATERIAL AND INSTALLATION NOT BY POLYGON.
6. VIBRATE CONCRETE FULL DEPTH OF FOUNDATION.
7. FOR DRILLED PIER FOUNDATIONS, PREVENT SOIL FROM ENTERING EXCAVATED HOLE (FORM, ETC).

4033 P. AZA, GOLDEN GATE CIRCLE
SUITE 10
CAMERON PARK, CA 95822
530.877.0016



poligon
PORTER
PLANNING & ARCHITECTURE

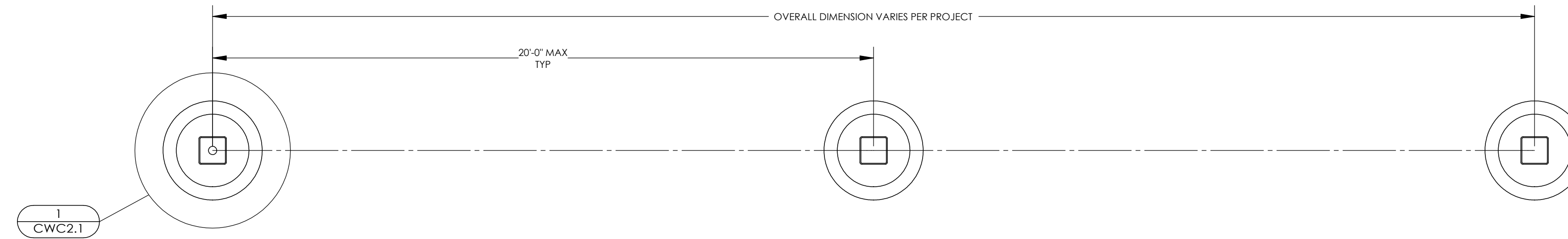


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A SEPARATE PROJECT
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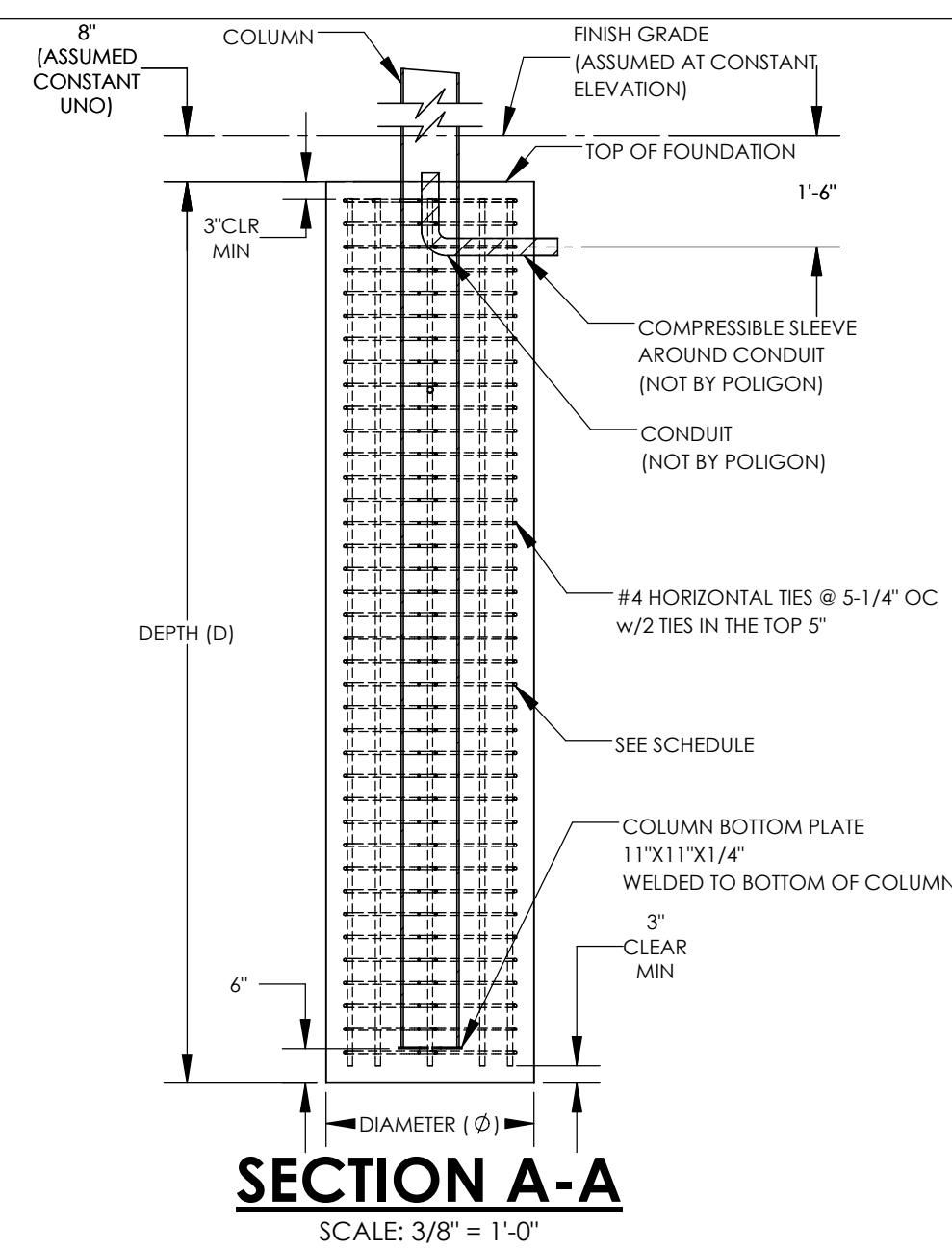
**FOUNDATION PLAN
DRILLED PIER**
WALKWAY COVER - CWC 10

CWC2.1



FOUNDATION PLAN (DRILLED PIER)

SCALE: 3/8" = 1'-0"



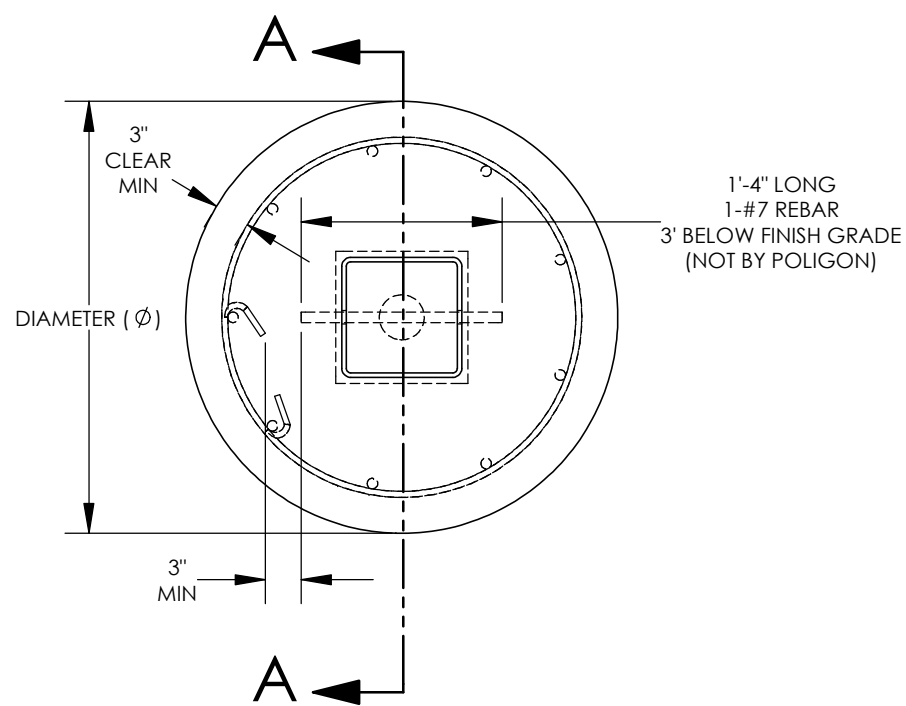
SECTION A-A

SCALE: 3/8" = 1'-0"

FOUNDATION REQUIREMENTS VARY PER PROJECT
SEE SHEET CWC1.0 FOR REQUIRED LOAD SCENARIO AND FOUNDATION TYPE (STEP 9 OF 'INSTRUCTIONS')
ONLY REFERENCE COPY OF PC DRAWINGS SUBMITTED FOR THIS PROJECT

DRILLED PIER SIZE AND REINFORCING REQUIREMENTS				
LOAD SCENARIO	DIAMETER (Ø)	DEPTH (D)	VERTICAL REINFORCING ¹	
			QTY	SIZE
1	3'-0"	13'-6"	9	#7
2	3'-0"	15'-0"	9	#7

¹ EQUALLY SPACED AROUND DRILLED PIER



DETAIL 1

SCALE: 3/4" = 1'-0"

FOUNDATION PLAN NOTES:

1. TOP OF ALL FOUNDATIONS MUST BE CONSTRUCTED AT ONE COMMON ELEVATION (COORDINATE WITH SITE PLANS - NOT BY POLYGON)
2. ALL FOUNDATIONS MUST BE CENTERED UNDER COLUMNS (UNO).
3. SEE SHEET CWC1.1 FOR CONCRETE REQUIREMENTS.
4. PRIOR TO FORMING AND CASTING FOUNDATIONS, REVIEW FOUNDATION PLAN FOR REQUIRED ORIENTATION.
5. FOUNDATION MATERIAL AND INSTALLATION NOT BY POLYGON.
6. VIBRATE CONCRETE FULL DEPTH OF FOUNDATION.

1035 PLAZA, GOLDEN GATE DISTRICT
SUITE 11
CAMERON PARK, CA 95822
530.877.0016



poligon
PORTER
CONSTRUCTION



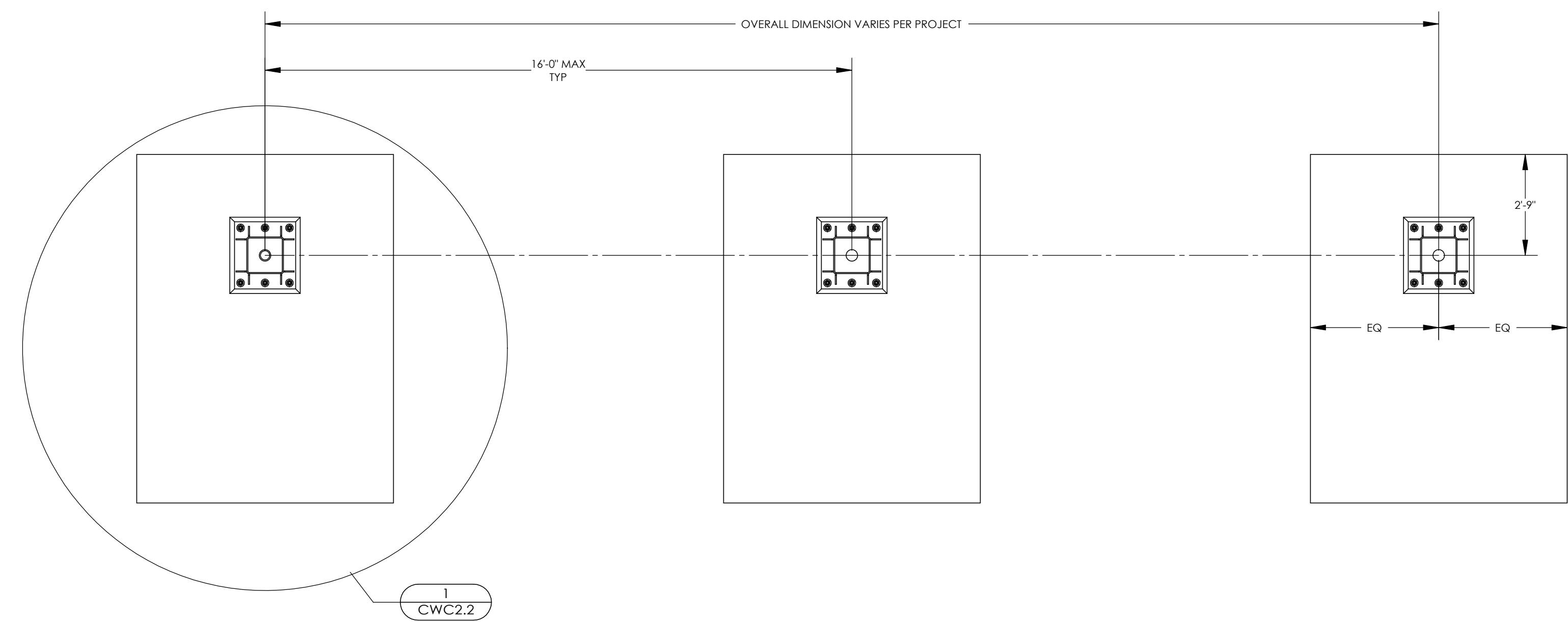
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DATE: 7/14/2023

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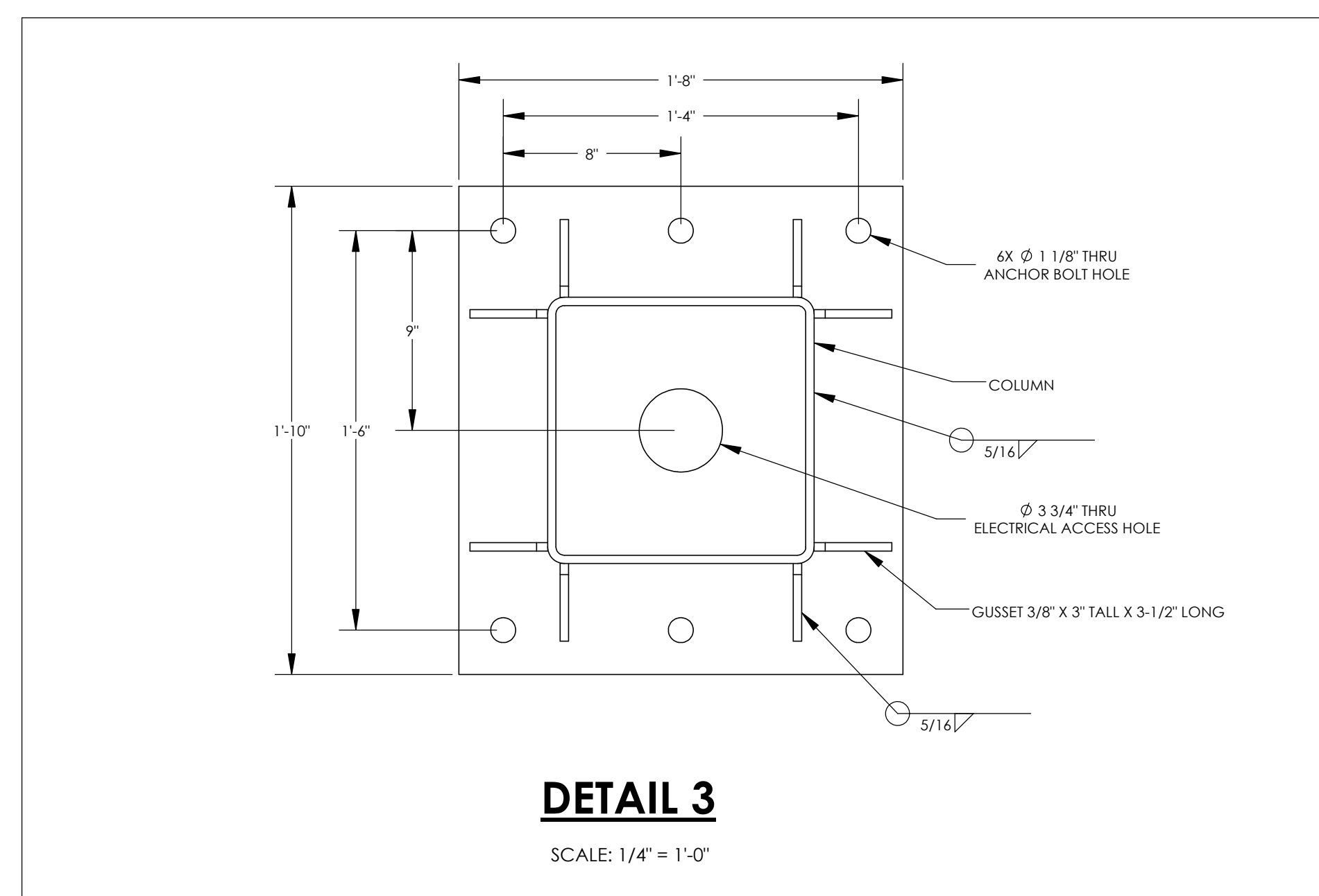
FOUNDATION PLAN SPREAD PAD

WALKWAY COVER - CWC 15

CWC2.2



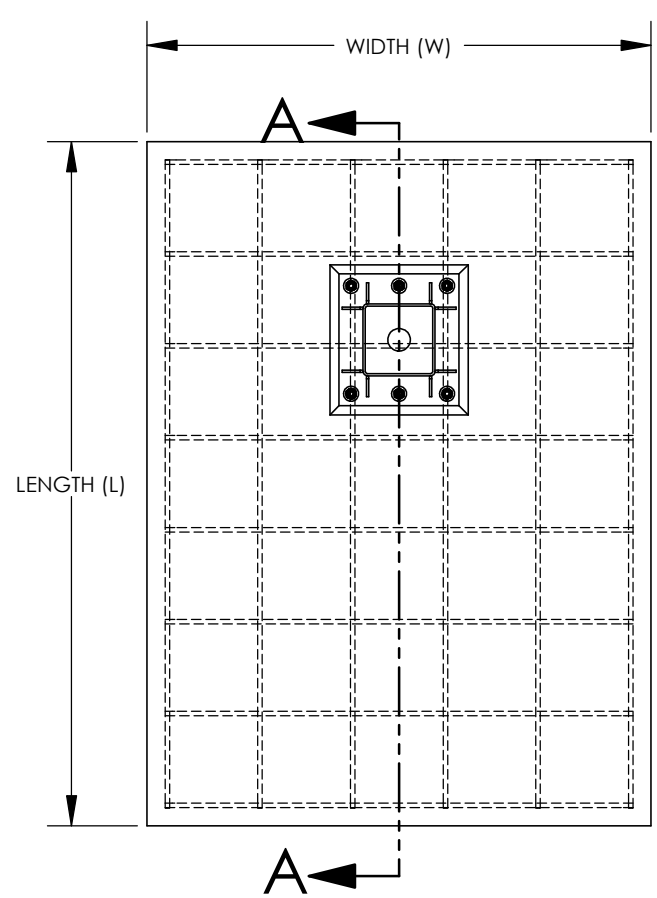
FOUNDATION PLAN (SPREAD PAD)
SCALE: 3/8" = 1'-0"



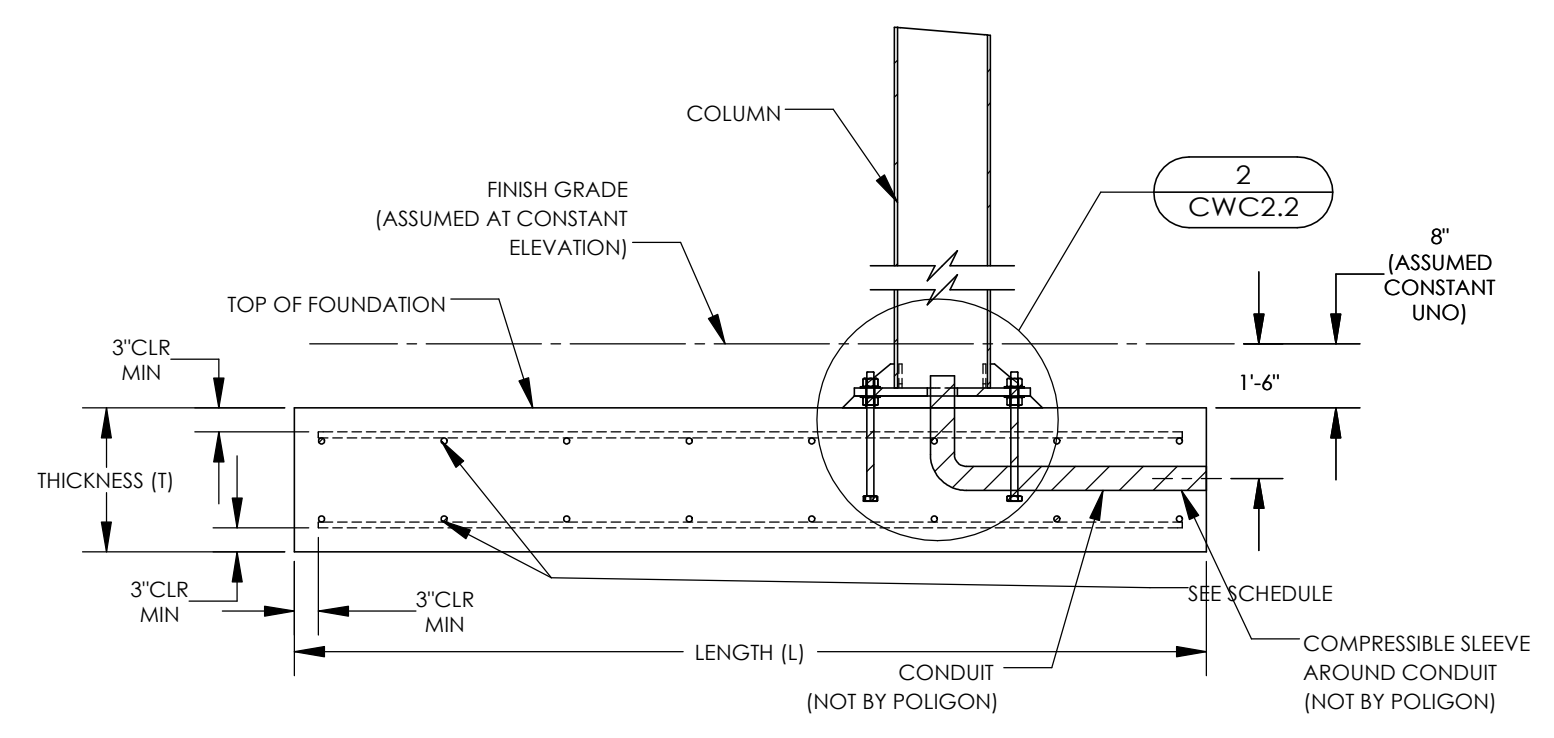
DETAIL 3
SCALE: 1/4" = 1'-0"

COLUMN BASEPLATE

3



DETAIL 1
SCALE: 3/8" = 1'-0"



SECTION A-A
SCALE: 3/8" = 1'-0"

FOUNDATION REQUIREMENTS VARY PER PROJECT
SEE SHEET CWC 1.0 FOR REQUIRED LOAD SCENARIO AND FOUNDATION TYPE (STEP 9 OF 'INSTRUCTIONS')
ONLY REFERENCE COPY OF PC DRAWINGS SUBMITTED FOR THIS PROJECT

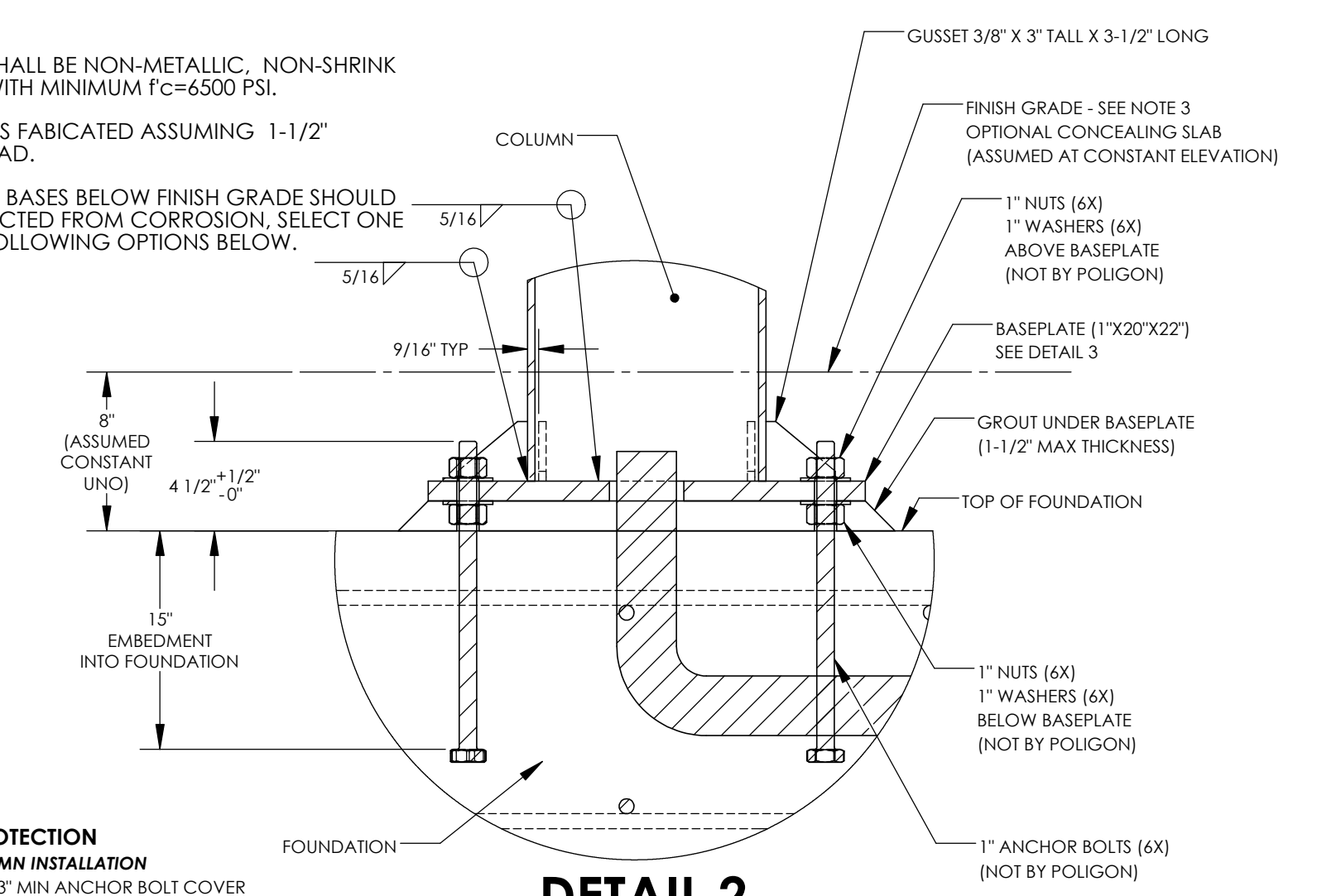
LOAD SCENARIO	WIDTH (W)	LENGTH (L)	THICKNESS (T)	HORIZONTAL REINFORCING ¹	
				QTY	SIZE
				1	8'-6"
2	9'-6"	11'-6"	1'-6"	11	#6

¹ EQUALLY SPACED EACH WAY, TOP AND BOTTOM

NOTES:

1. GROUT SHALL BE NON-METALLIC, NON-SHRINK GROUT WITH MINIMUM F_c=6500 PSI.
2. COLUMNS FABRICATED ASSUMING 1-1/2" GROUT PAD.
3. COLUMN BASES BELOW FINISH GRADE SHOULD BE PROTECTED FROM CORROSION, SELECT ONE OF THE FOLLOWING OPTIONS BELOW:

COLUMN BASE PROTECTION BELOW SURFACE COLUMN INSTALLATION
 [] CONCRETE SLAB - 3" MIN ANCHOR BOLT COVER
 [] MASTIC COATING - 1/4" THICK MIN COATING ON ALL STEEL SURFACES BELOW GRADE



DETAIL 2
SCALE: 1-1/2" = 1'-0"

COLUMN BASEPLATE AND ANCHOR BOLTS

2

SPREAD PAD FOUNDATION

1

FOUNDATION PLAN NOTES:

1. TOP OF ALL FOUNDATIONS MUST BE CONSTRUCTED AT ONE COMMON ELEVATION (COORDINATE WITH SITE PLANS - NOT BY POLYGON)
2. ALL FOUNDATIONS MUST BE CENTERED UNDER COLUMNS (UNO).
3. SEE SHEET CWC1.1 FOR CONCRETE REQUIREMENTS.
4. PRIOR TO FORMING AND CASTING FOUNDATIONS, REVIEW FOUNDATION PLAN FOR REQUIRED ORIENTATION.
5. FOUNDATION MATERIAL AND INSTALLATION NOT BY POLYGON.
6. VIBRATE CONCRETE FULL DEPTH OF FOUNDATION.
7. FOR DRILLED PIER FOUNDATIONS, PREVENT SOIL FROM ENTERING EXCAVATED HOLE (FORM, ETC).

1033 P. AZA, GOLDEN GATE DISTRICT
SUITE 11
CAMERON PARK, CA 95822
530.877.0016



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PORTER
PLANNING & ENGINEERING

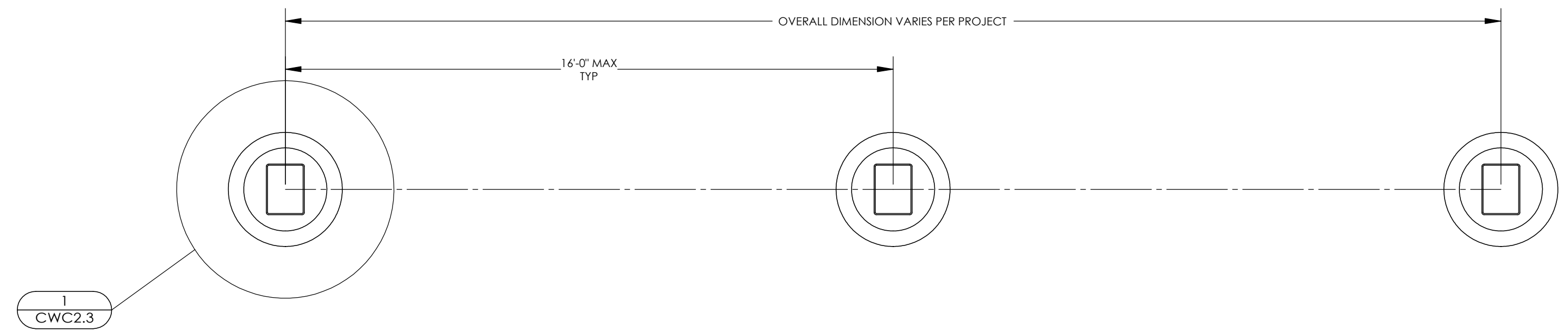


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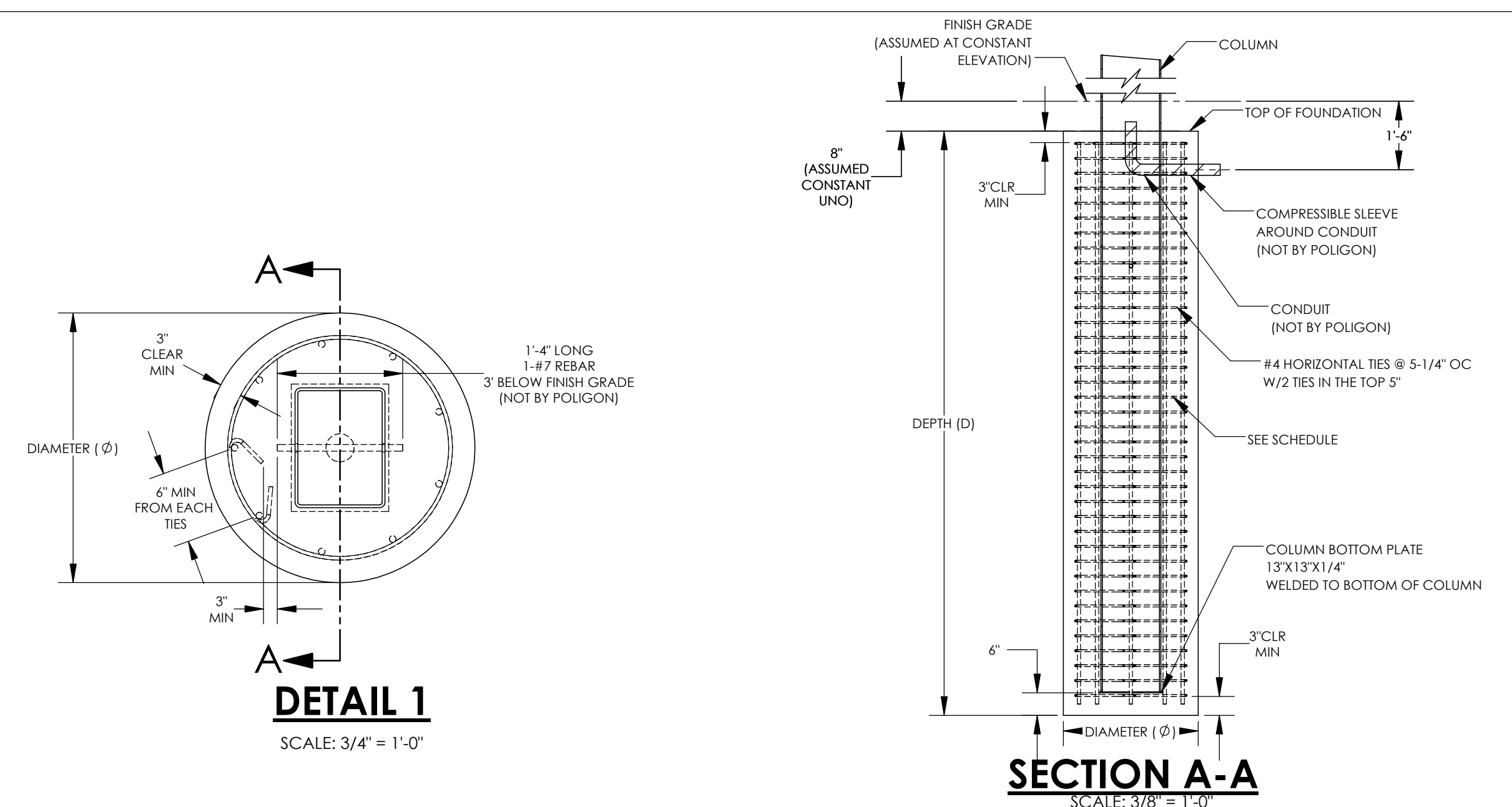
**PRE-CHECK (PC)
DOCUMENT**
CODE: 2022 CBC
A SEPARATE PROJECT
APPLICATION FOR
CONSTRUCTION IS REQUIRED.

**FOUNDATION PLAN
DRILLED PIER**
WALKWAY COVER - CWC 15

CWC2.3



FOUNDATION PLAN (DRILLED PIER)
SCALE: 3/8" = 1'-0"



FOUNDATION REQUIREMENTS VARY PER PROJECT
SEE SHEET CWC 1.0 FOR REQUIRED LOAD SCENARIO AND FOUNDATION TYPE (STEP 9 OF 'INSTRUCTIONS')
ONLY REFERENCE COPY OF PC DRAWINGS SUBMITTED FOR THIS PROJECT

LOAD SCENARIO	DIAMETER (Ø)	DEPTH (D)	VERTICAL REINFORCING ¹	
			QTY	SIZE
1	3'-0"	15'-0"	9	#7
2	3'-0"	17'-0"	9	#7

¹ EQUALLY SPACED AROUND DRILLED PIER

SECTION A-A
SCALE: 3/8" = 1'-0"

DRILLED PIER FOUNDATION



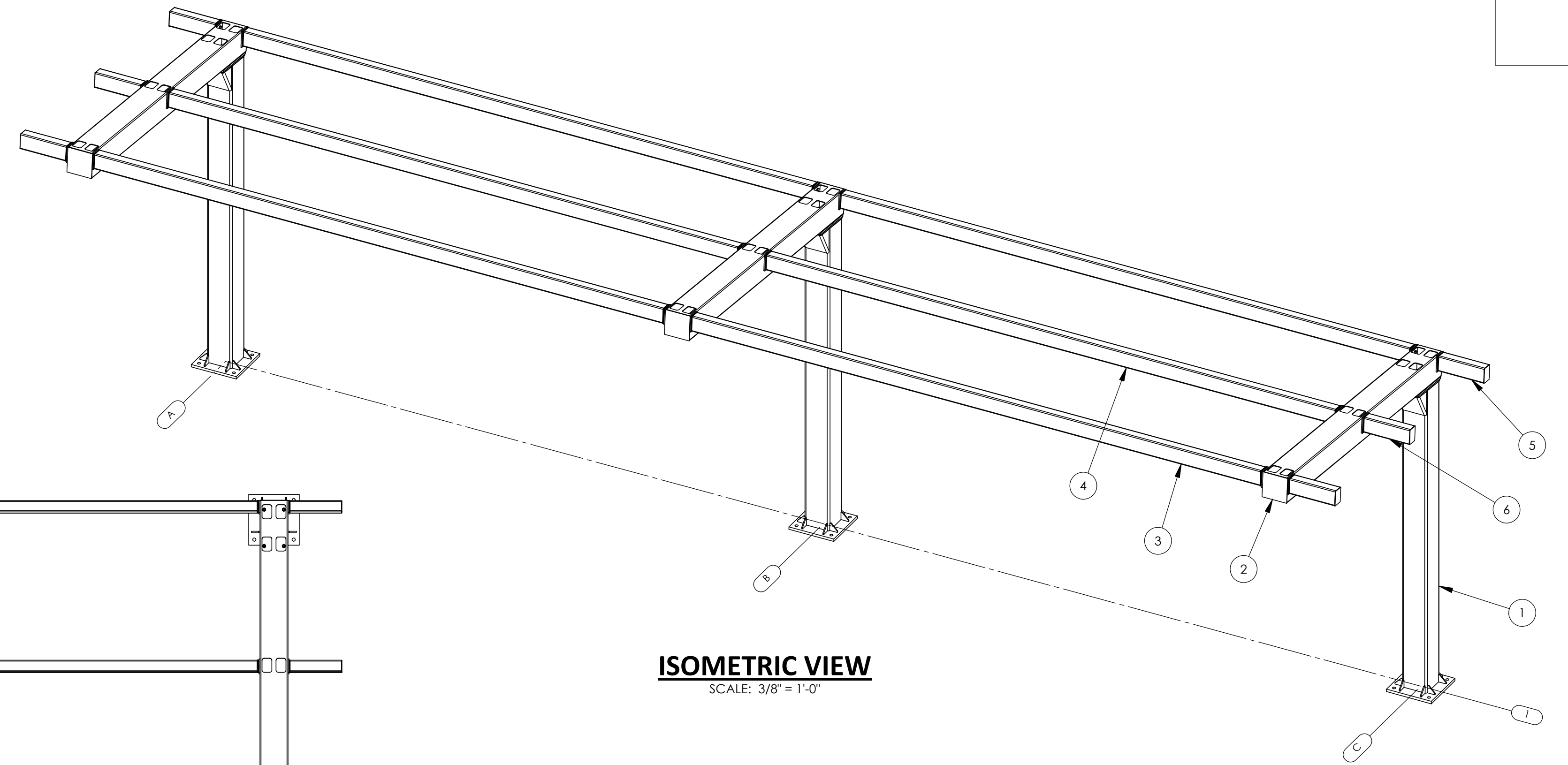
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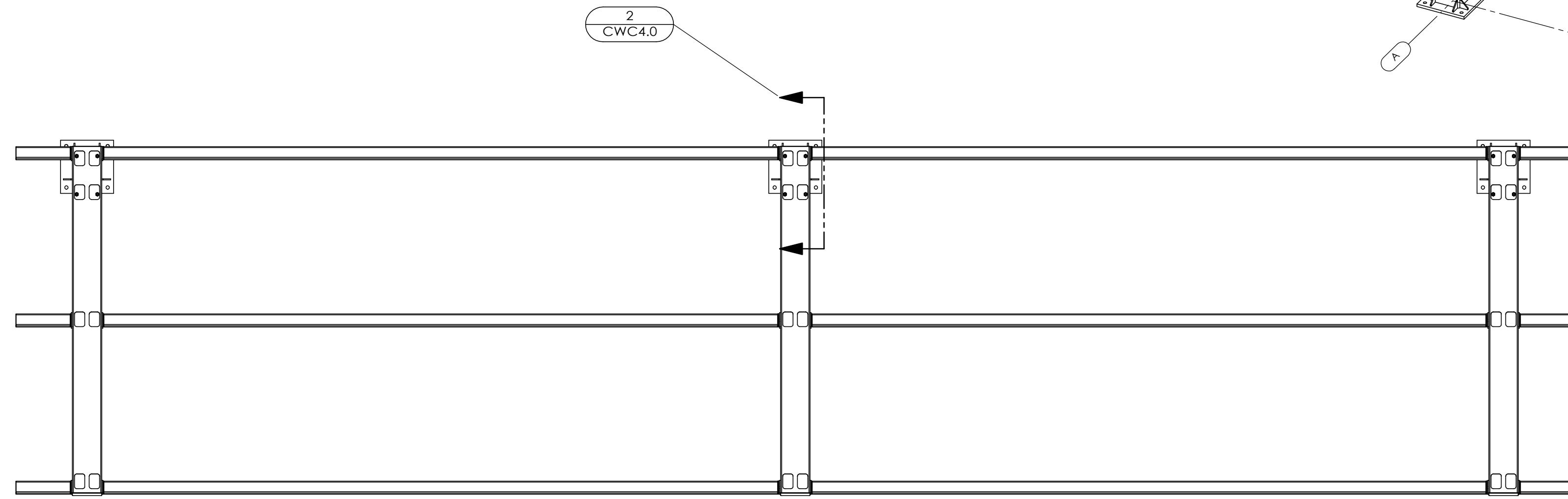
FRAMING PLAN

WLAKWAY COVER - CWC 10

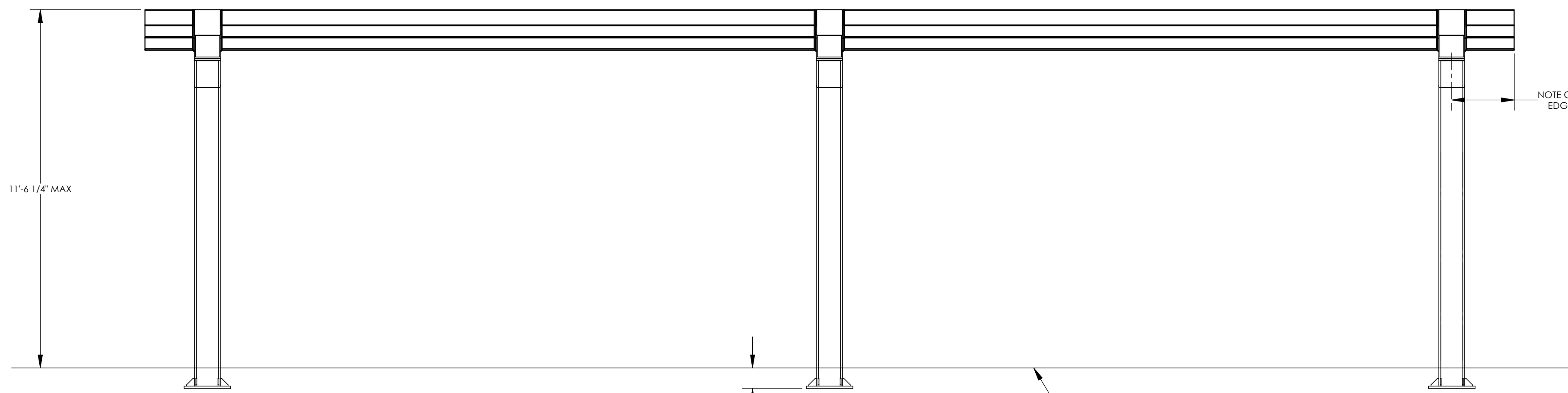
CWC3.0



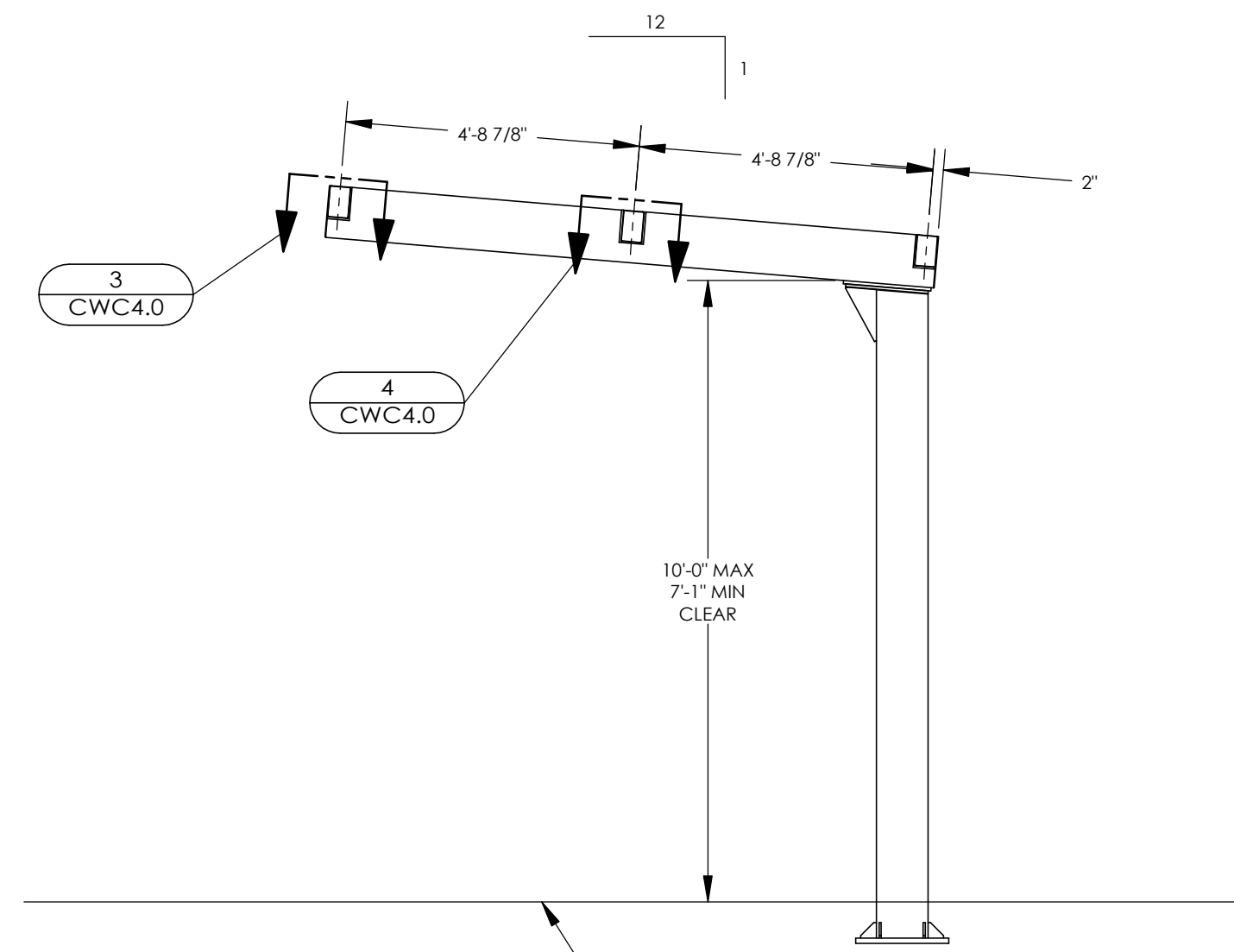
ISOMETRIC VIEW
SCALE: 3/8" = 1'-0"



PLAN VIEW
SCALE: 3/8" = 1'-0"



FRONT ELEVATION
SCALE: 3/8" = 1'-0"



SIDE ELEVATION
SCALE: 3/8" = 1'-0"

ITEM	QTY.	PART NO.	DESCRIPTION	MATERIAL	WEIGHT
6	2	-	PURLIN TAIL ASM	HSS6X4X3/16	21.39
5	4	-	EBEAM TAIL ASM	HSS6X4X3/16	21.39
4	2	-	PURLIN ASM	HSS6X4X3/16	231.20
3	4	-	EBEAM ASM	HSS6X4X3/16	231.20
2	3	-	GABLE BEAM_1 ASM	HSS10X10X3/16	287.49
1	3	-	COL_1 ASM	HSS10X10X3/8	632.65

11'-6 1/4" MAX

2'-0"
NOTE COLUMN CENTERLINE TO
EDGE OF TAIL SEE CWC5.0
EDGE OF ROOF

SEE DETAIL 2
SHEET CWC2.0/CWC2.1

FINISH GRADE
(ASSUMED AT CONSTANT
ELEVATION UNLESS
OTHERWISE NOTED)

FINISH GRADE
(ASSUMED AT CONSTANT
ELEVATION UNLESS
OTHERWISE NOTED)

10'-0" MAX
7'-1" MIN
CLEAR

STATE APPROVALS-SITE

4855 P. AZA, GOLDEN RIDGE, DUBLIN, CA 94568
 SUITE 101
 CHAMBERLAIN TOWN CENTER, CA 94588
 925.877.6016

poligon
 PORTER & PORTER
 ARCHITECTS

REGISTERED PROFESSIONAL ENGINEER
 JESSICA E. HANFERT
 No. 55476
 STRUCTURAL
 STATE OF CALIFORNIA

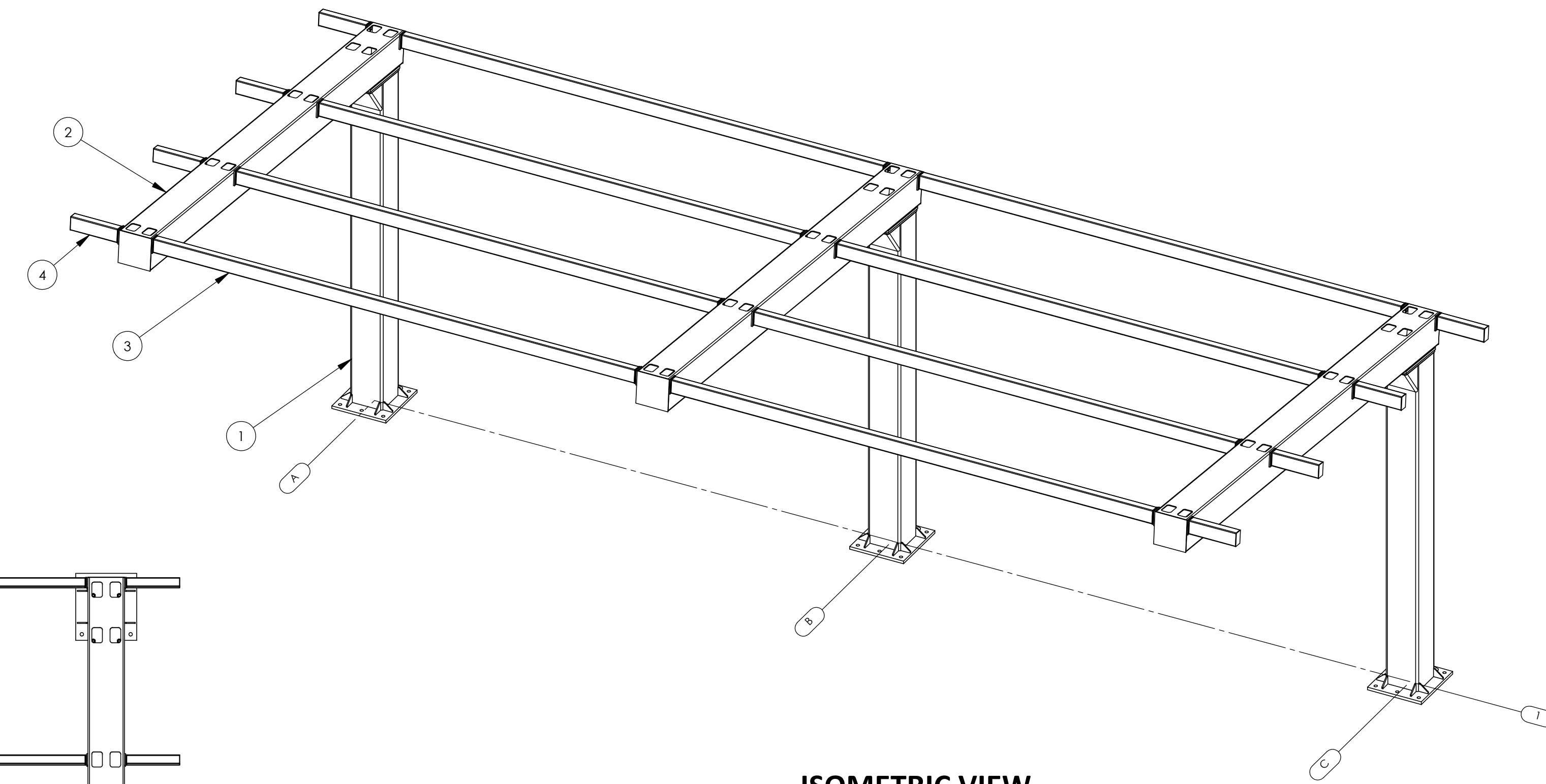
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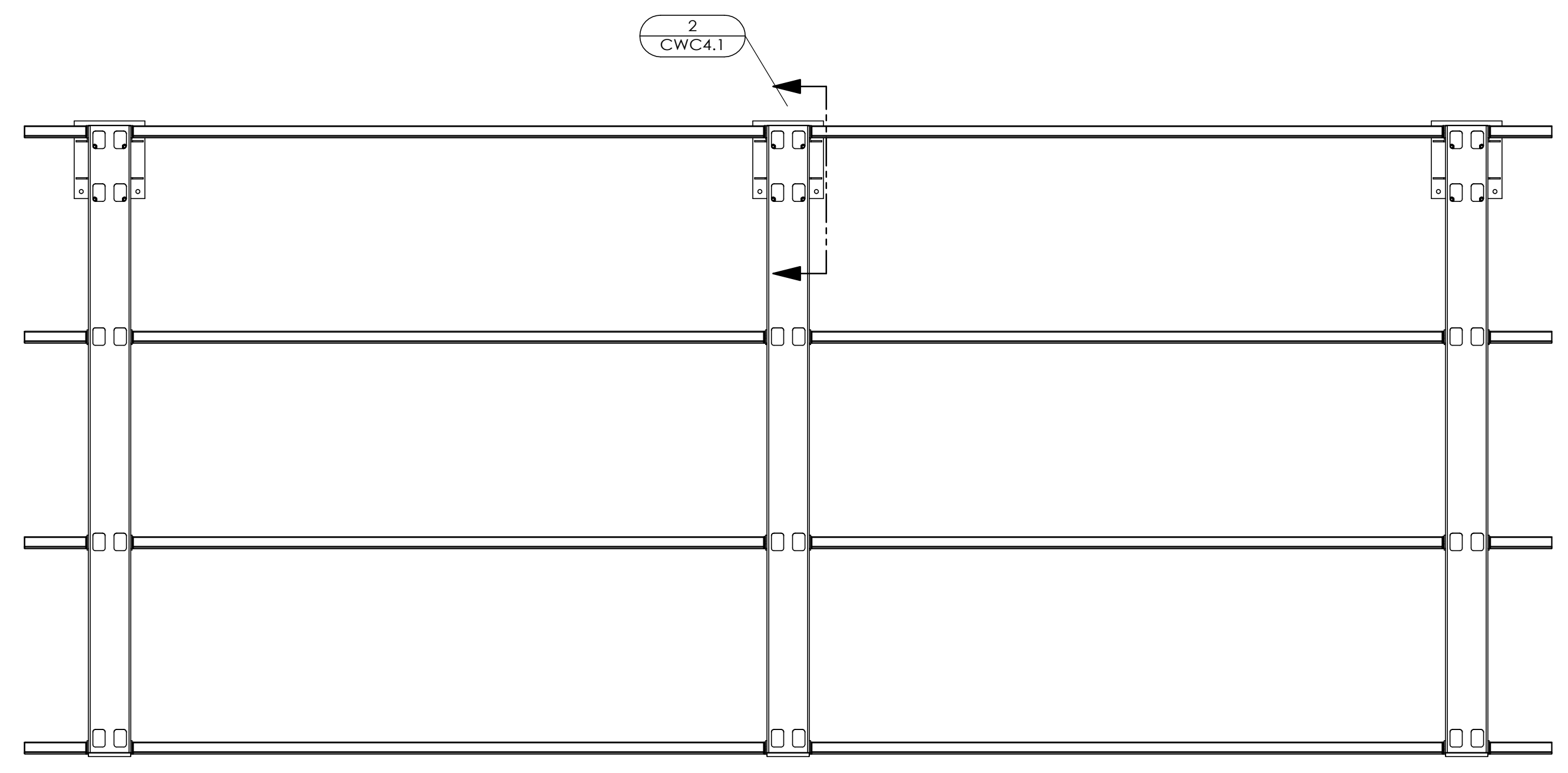
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FRAMING PLAN
 WALKWAY COVER - CWC 15

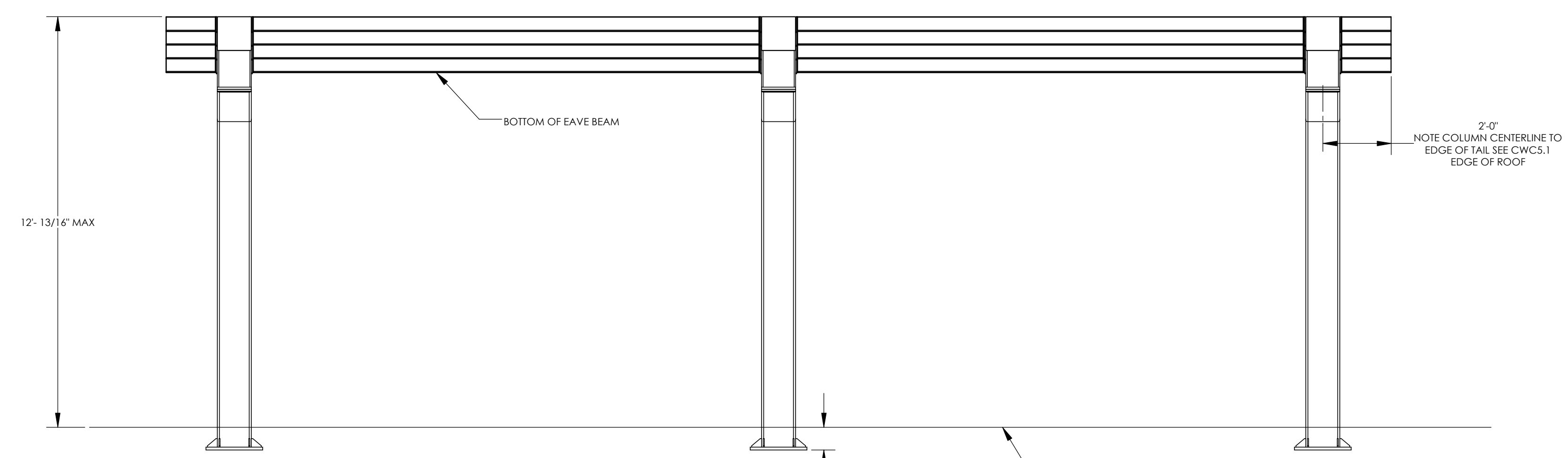
CWC3.1



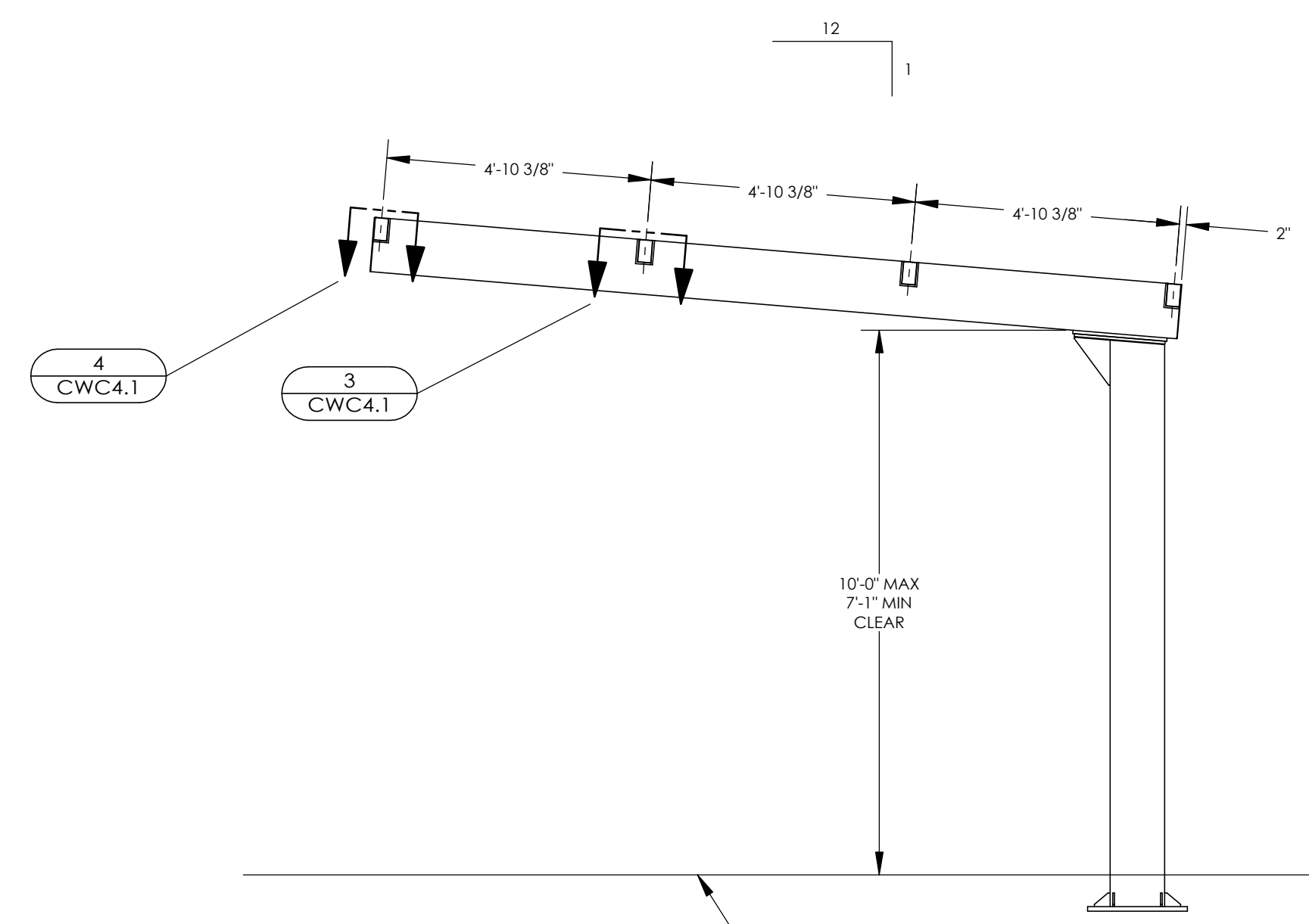
ISOMETRIC VIEW
 SCALE: 3/8" = 1'-0"



PLAN VIEW
 SCALE: 3/8" = 1'-0"



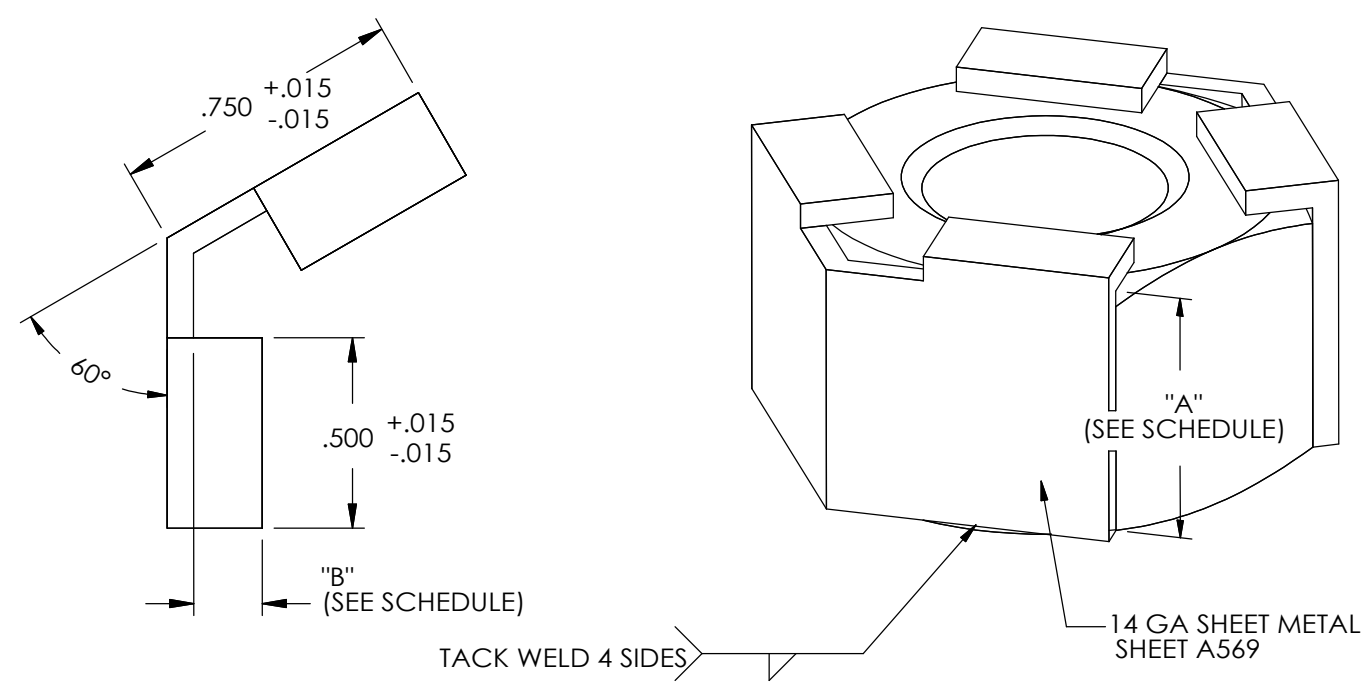
FRONT ELEVATION
 SCALE: 3/8" = 1'-0"



SIDE ELEVATION
 SCALE: 3/8" = 1'-0"

ITEM	QTY.	PART NO.	DESCRIPTION	MATERIAL	WEIGHT
4	8	-	PURLIN TAIL ASM	HSS6X4X1/8	11.25
3	8	-	PURLIN ASM	HSS6X4X1/8	98.31
2	3	-	GABLE BEAM_1 ASM	HSS12X12X1/4	646.58
1	3	-	COL_1 ASM	HSS12X12X3/8	797.01

ALL HIDDEN NUTS AND BOLTS (INSTALLED IN SHOP DURING FABRICATION) ARE SECURED WITH THIS NUT AND BOLT RESTAINING SYSTEM.

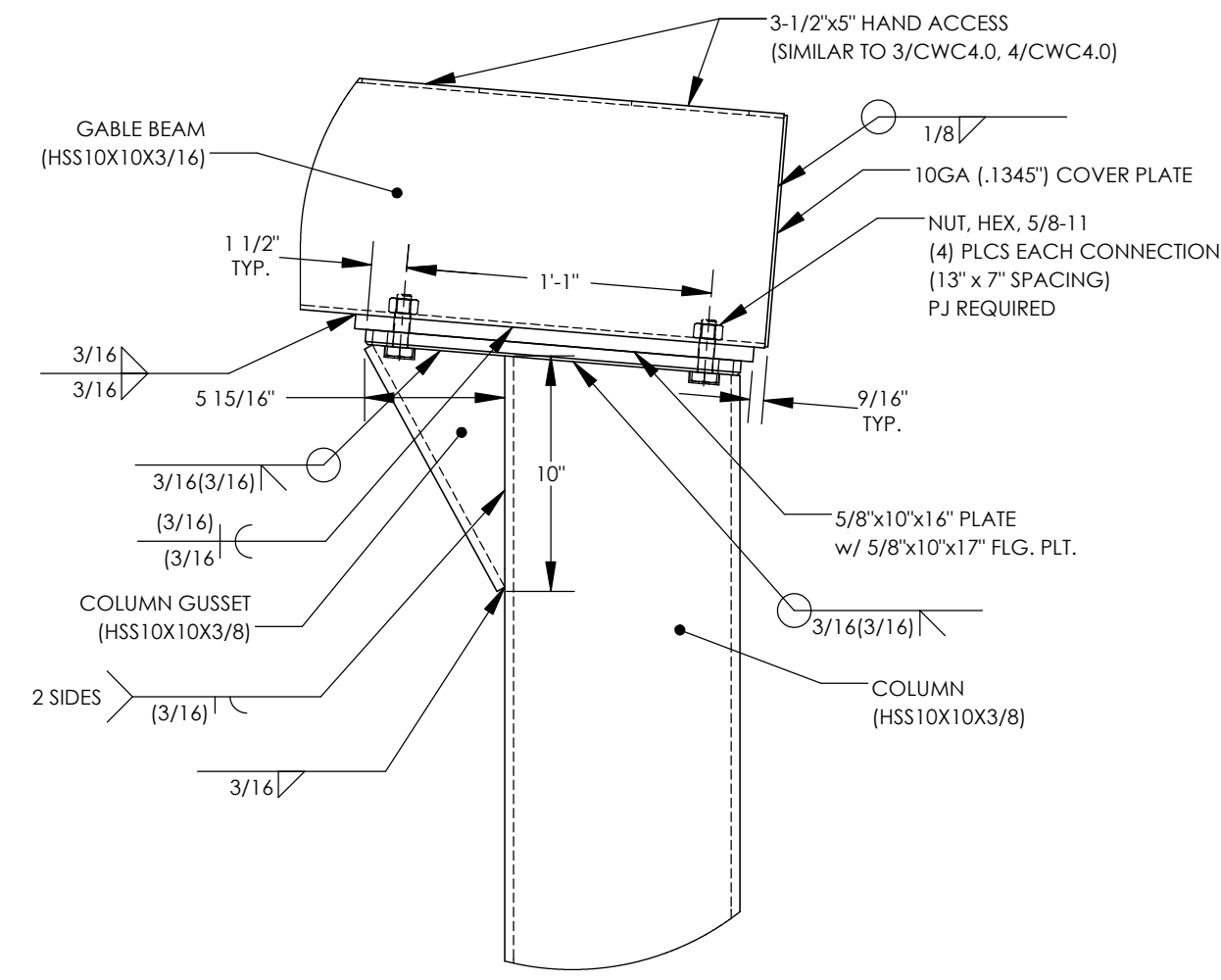


DIMENSION SCHEDULE

FASTENER	DIM A	DIM B
5/8" NUT	.631 +.000 -.015	.180 +.015 -.015
5/8" BOLT	.403 +.000 -.015	.250 +.015 -.015
3/4" NUT	.758 +.000 -.015	.180 +.015 -.015
3/4" BOLT	.483 +.000 -.015	.375 +.015 -.015
1" NUT	1.012 +.000 -.015	.180 +.015 -.015
1" BOLT	.643 +.000 -.015	.375 +.015 -.015

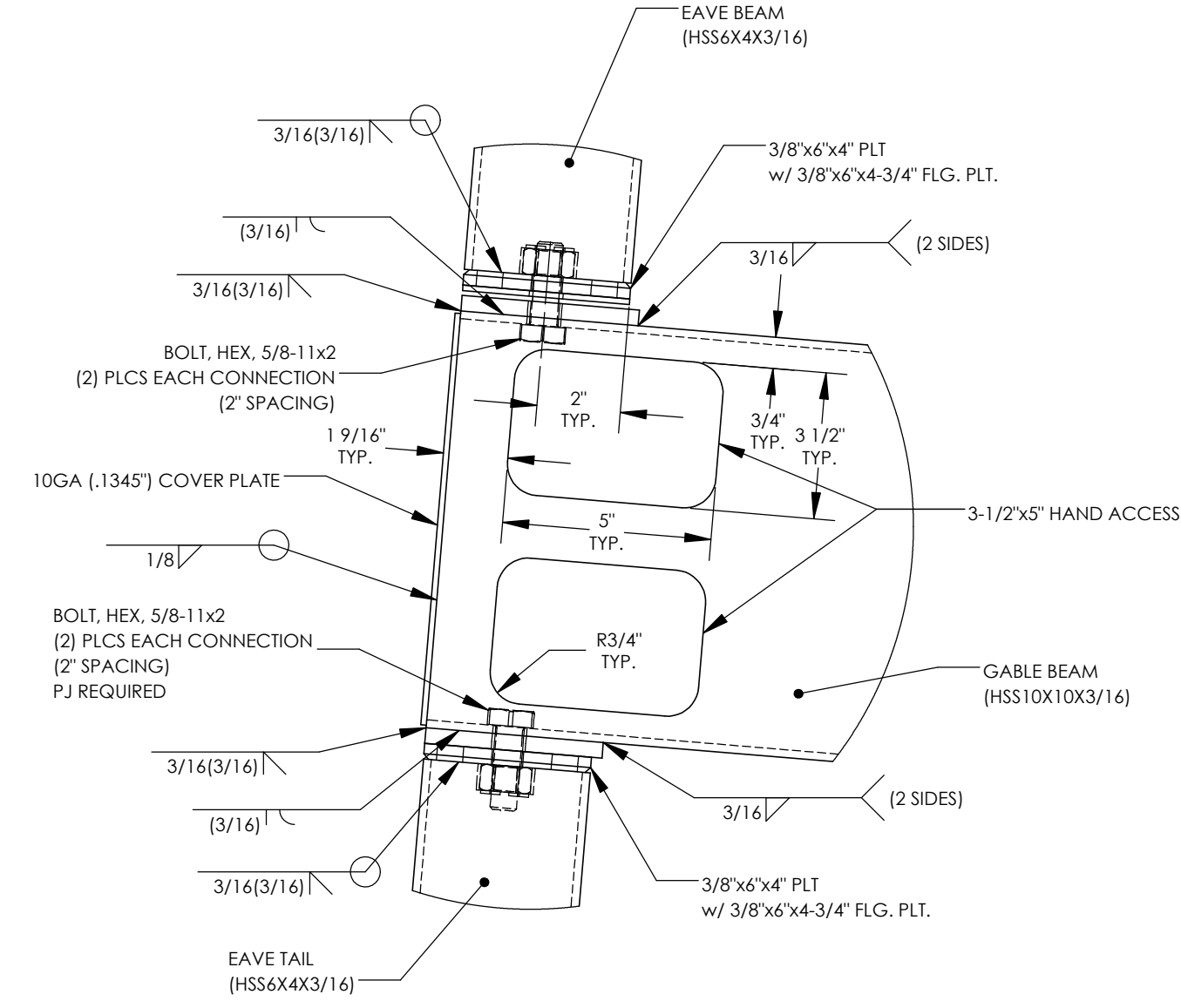
NUT & BOLT RESTRAINING SYSTEM

1



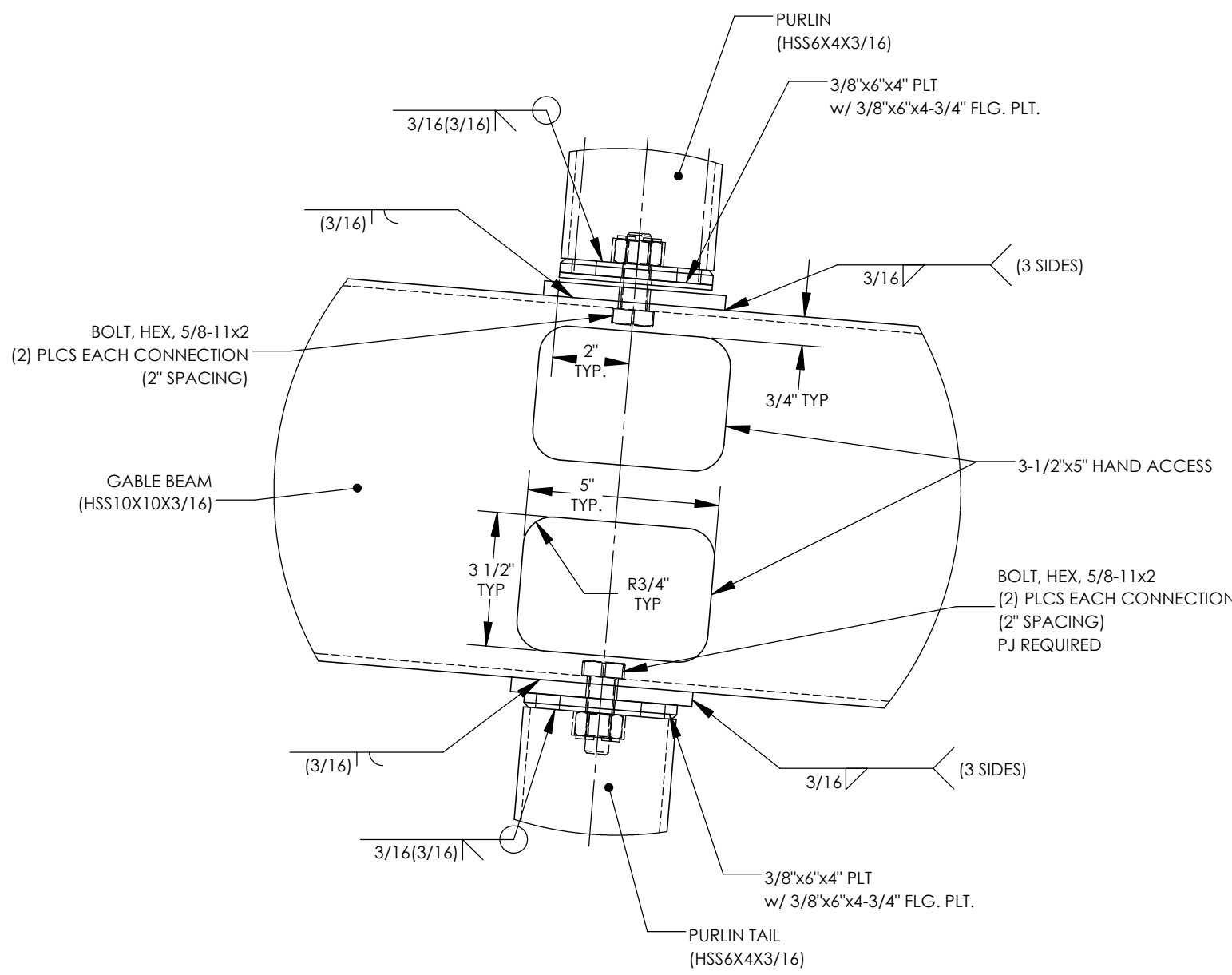
GABLE BEAM CONNECTION @ COLUMN

2



EAVE BEAM AND EAVE TAIL CONNECTION @ GABLE BEAM

3



PURLIN AND PURLIN TAIL CONNECTION @ GABLE BEAM

4

STATE APPROVALS-SITE

403 P. AZA, GOLDEN RICE
SUITE 11,
CHAMBERLIN RD, BERK, CA 94603
510.877.1016



poligon
PORTER
STRUCTURAL ENGINEERS



STATE APPROVALS-PC

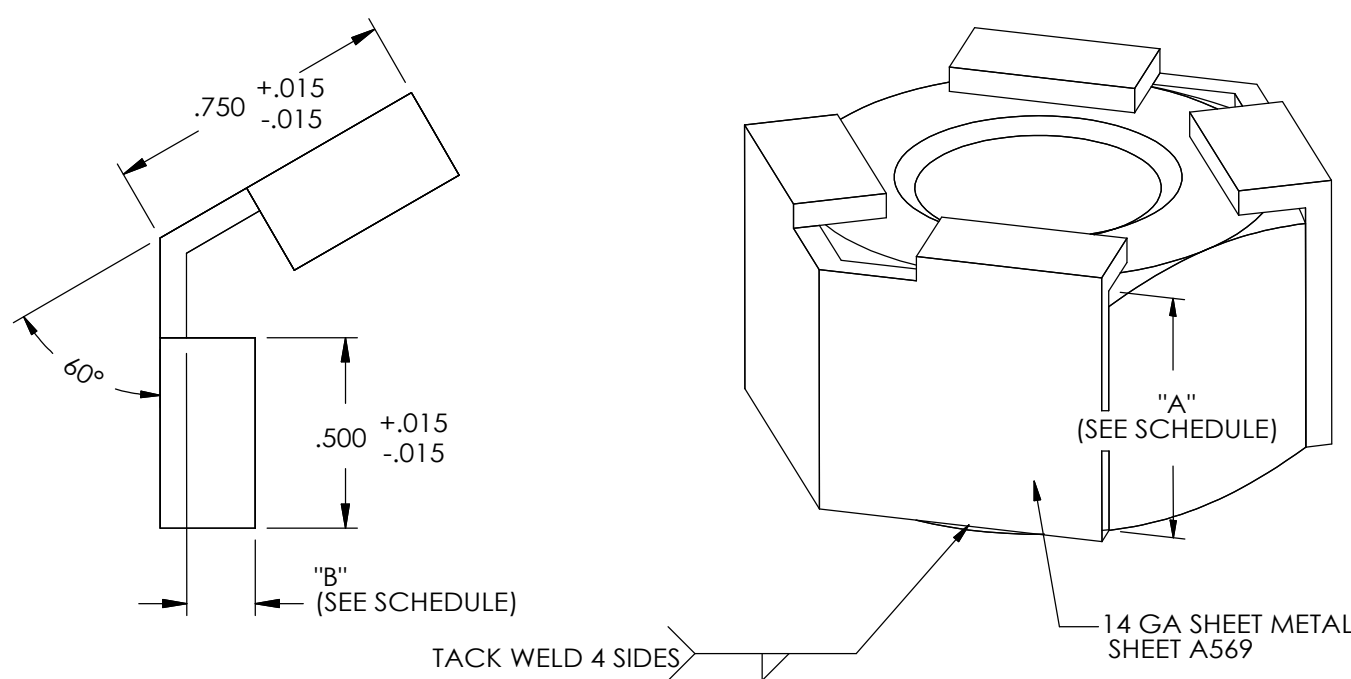
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FRAME
CONNECTION
DETAILS
WALKWAY COVER - CWC 10

CWC4.0

ALL HIDDEN NUTS AND BOLTS (INSTALLED IN SHOP DURING FABRICATION) ARE SECURED WITH THIS NUT AND BOLT RESTAINING SYSTEM.

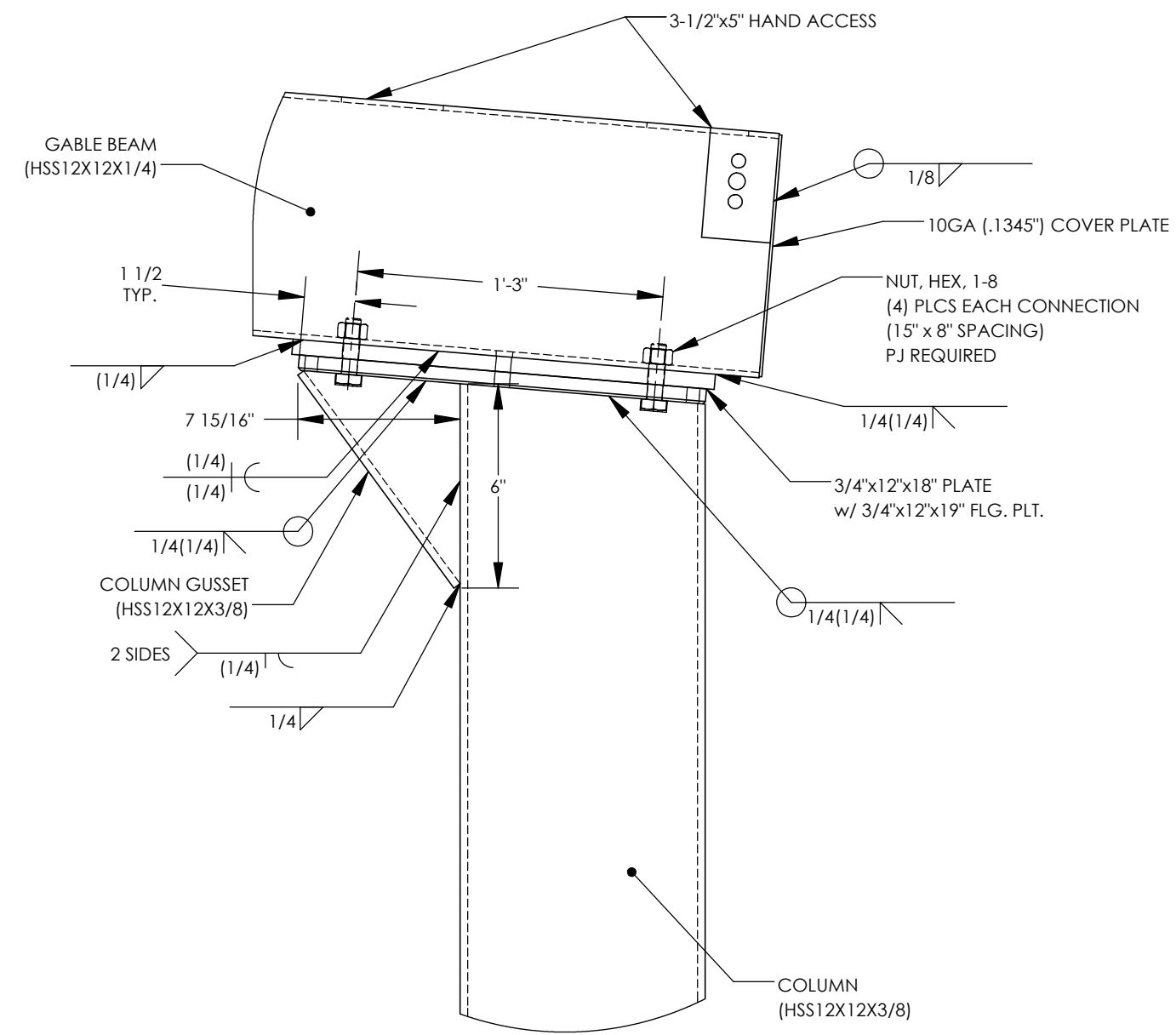


DIMENSION SCHEDULE

FASTENER	DIM A	DIM B
5/8" NUT	.631 +.000 -.015	.180 +.015 -.015
5/8" BOLT	.403 +.000 -.015	.250 +.015 -.015
3/4" NUT	.758 +.000 -.015	.180 +.015 -.015
3/4" BOLT	.483 +.000 -.015	.375 +.015 -.015
1" NUT	1.012 +.000 -.015	.180 +.015 -.015
1" BOLT	.643 +.000 -.015	.375 +.015 -.015

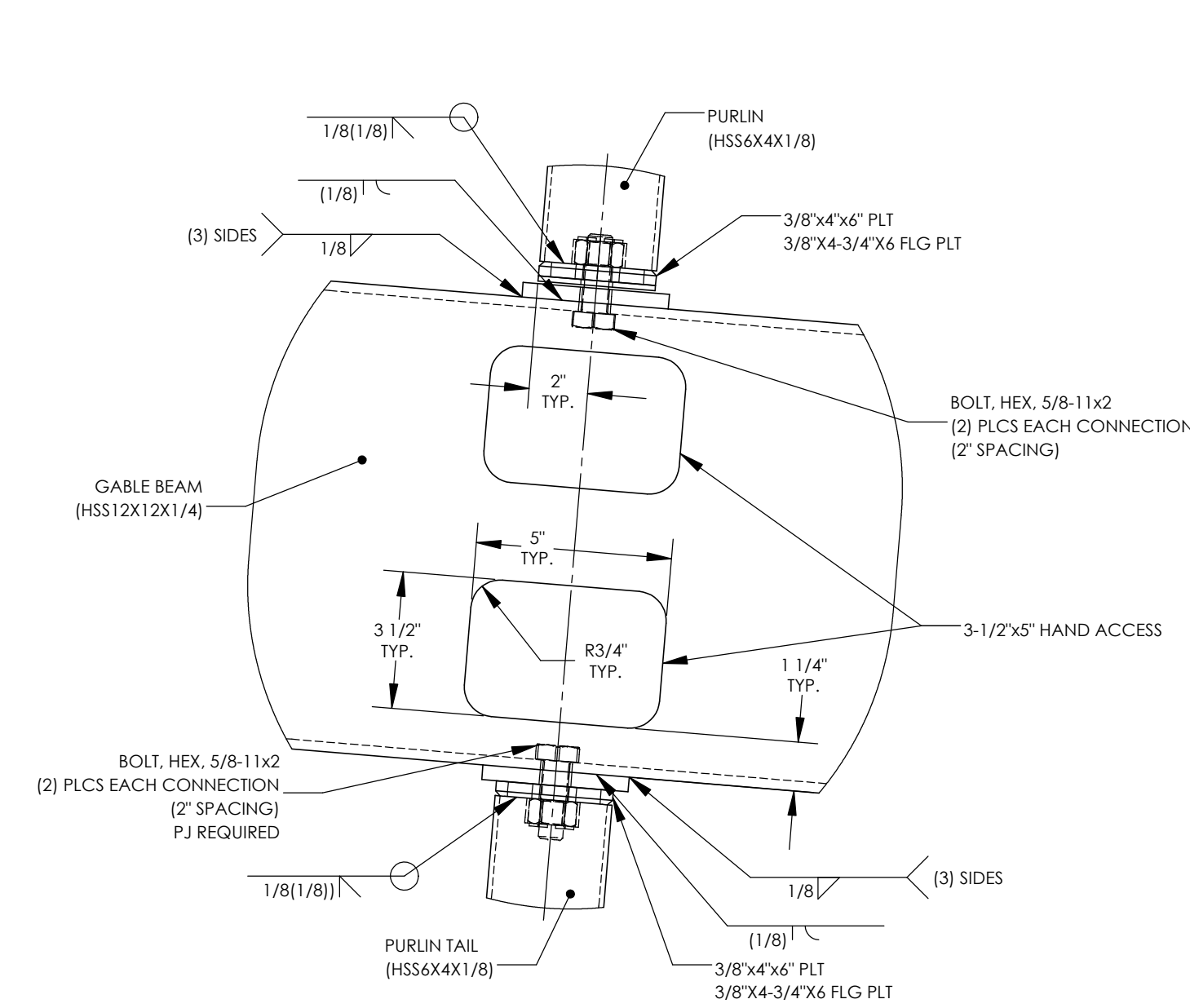
NUT & BOLT RESTRAINING SYSTEM

1



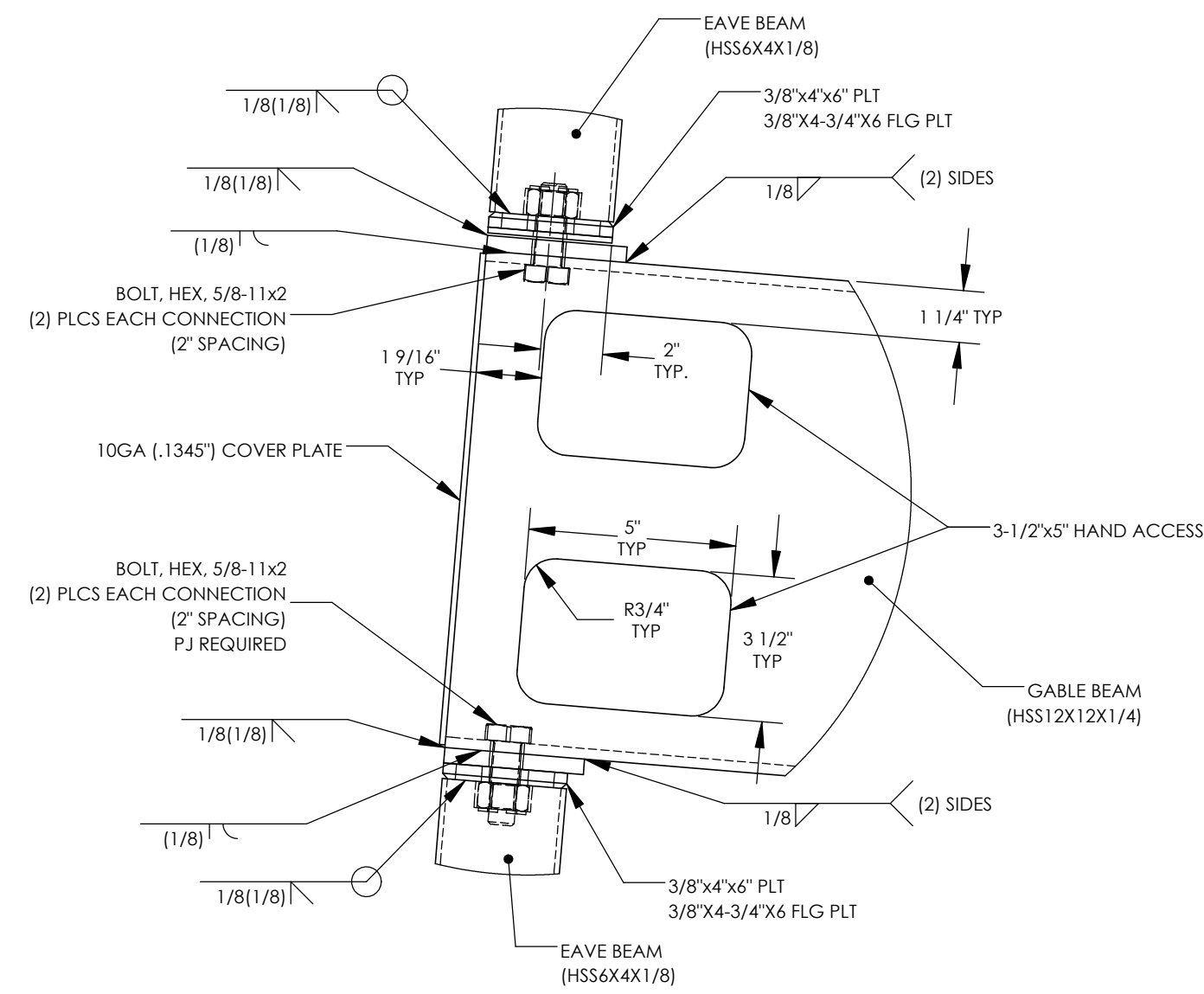
GABLE BEAM CONNECTION @ COLUMN

2



PURLIN AND PURLIN TAIL CONNECTION @ GABLE BEAM

3



EAVE AND EAVE TAIL CONNECTION @ GABLE BEAM

4

STATE APPROVALS-SITE

4083 P. AZA, GOLDEN RICE
SUITE B,
CHAMPION TOWER BLDG. 24, 95828
530 BLYWATER



STATE APPROVALS-PC

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FRAME
CONNECTION
DETAILS
WALKWAY COVER - CWC 15

CWC4.1

4055 P. AZA, GOLDEN GATE CIRCLE
SUITE 11
CAMERON PARK, CA 95822
(916) 777-1010



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ARCHITECTS



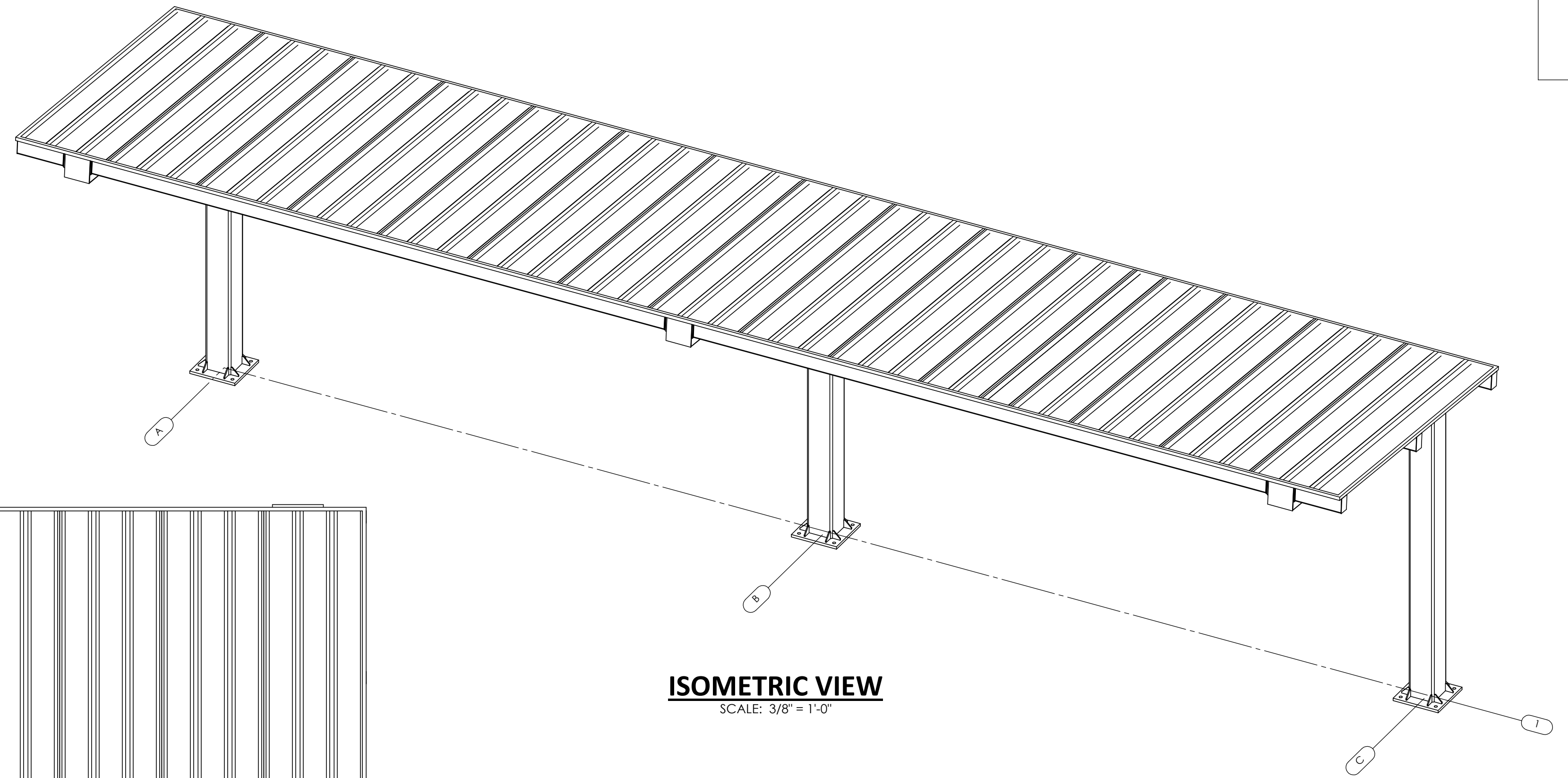
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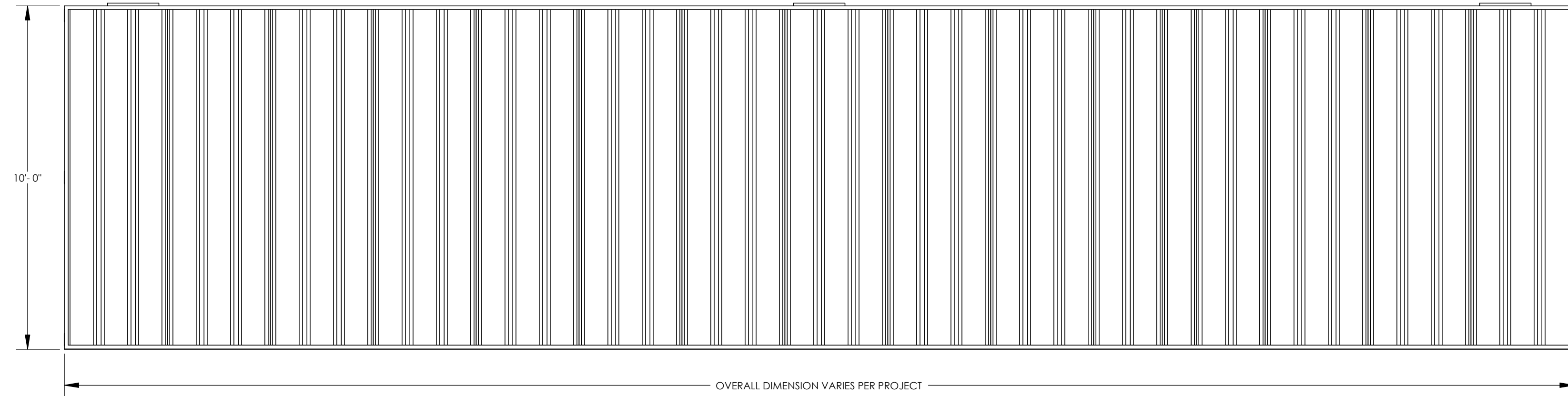
ARCHITECTURAL
VIEWS

WALKWAY COVER - CWC 10

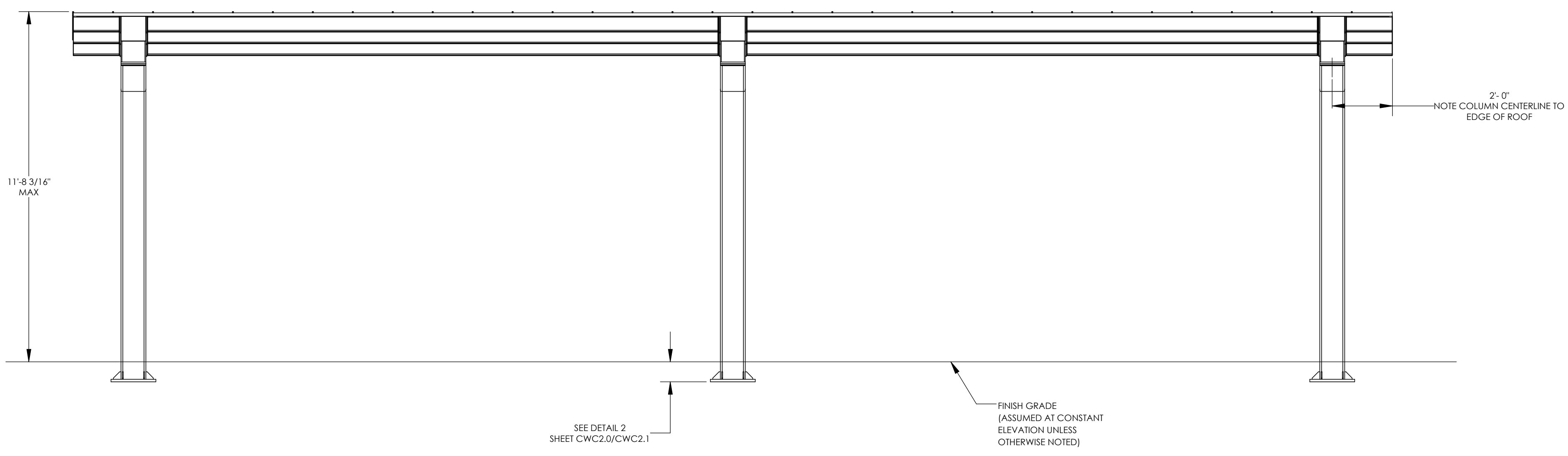
CWC5.0



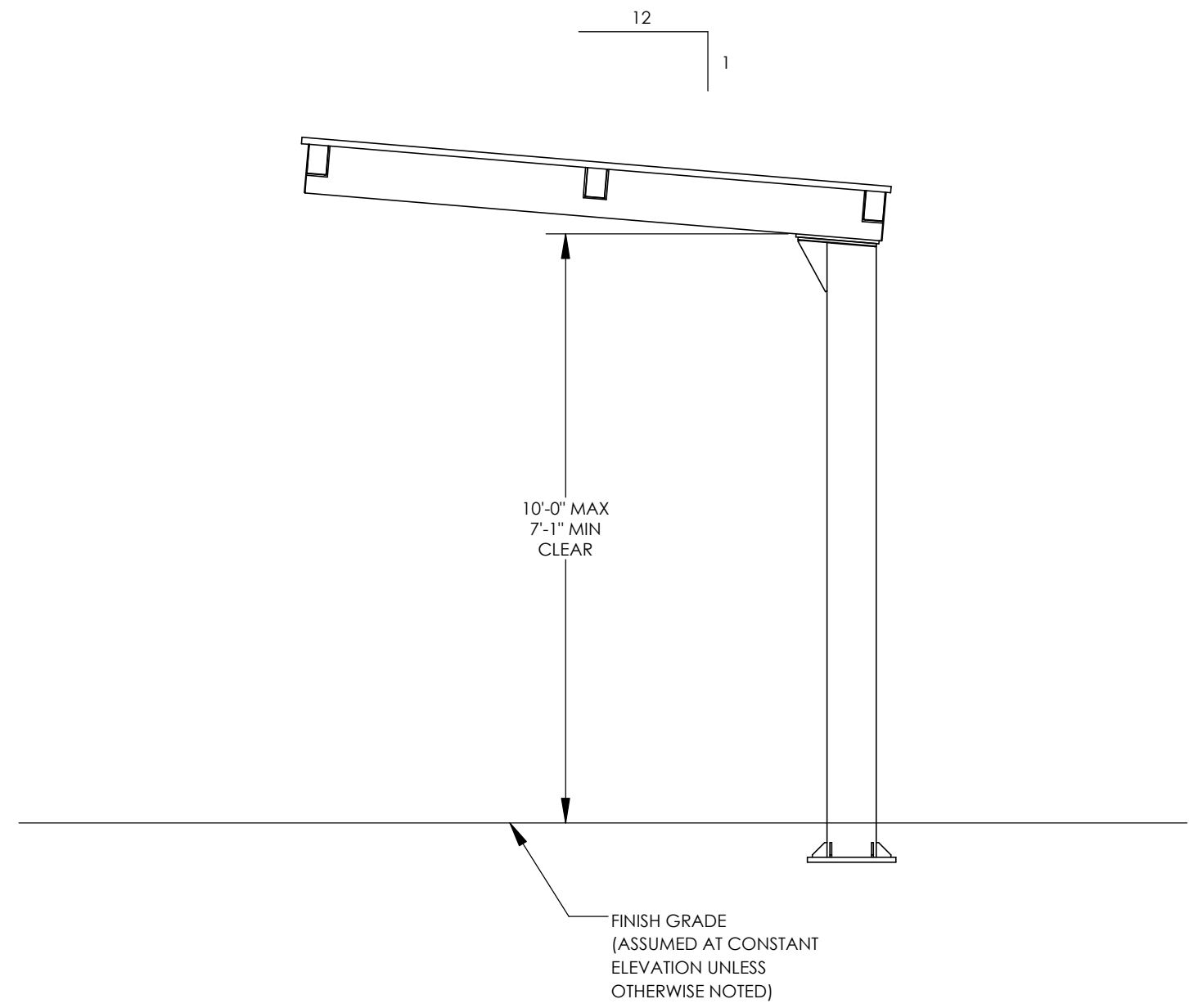
ISOMETRIC VIEW
SCALE: 3/8" = 1'-0"



PLAN VIEW
SCALE: 3/8" = 1'-0"



FRONT ELEVATION
SCALE: 3/8" = 1'-0"



SIDE ELEVATION
SCALE: 3/8" = 1'-0"

11'-8 3/16" MAX

SEE DETAIL 2 SHEET CWC2.0/CWC2.1

FINISH GRADE (ASSUMED AT CONSTANT ELEVATION UNLESS OTHERWISE NOTED)

2'-0" NOTE COLUMN CENTERLINE TO EDGE OF ROOF

10'-0" MAX 7'-1" MIN CLEAR

FINISH GRADE (ASSUMED AT CONSTANT ELEVATION UNLESS OTHERWISE NOTED)

OVERALL DIMENSION VARIES PER PROJECT

4033 P. AZA, GOLDEN GATE CIRCLE
SUITE 11
CAMERON PARK, CA 95822
530.877.0016



poligon
PORTER
PLANNING & ARCHITECTURE

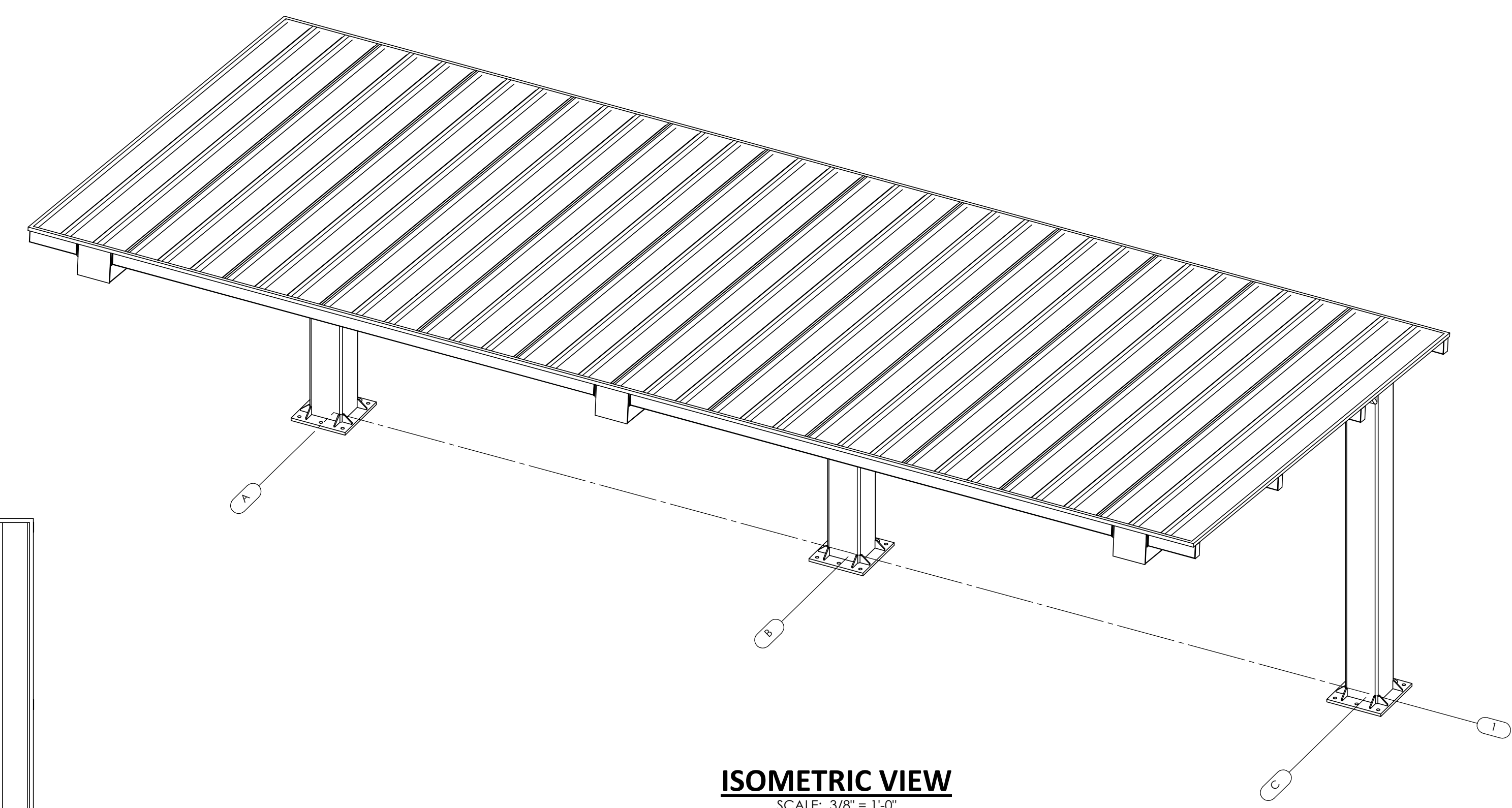


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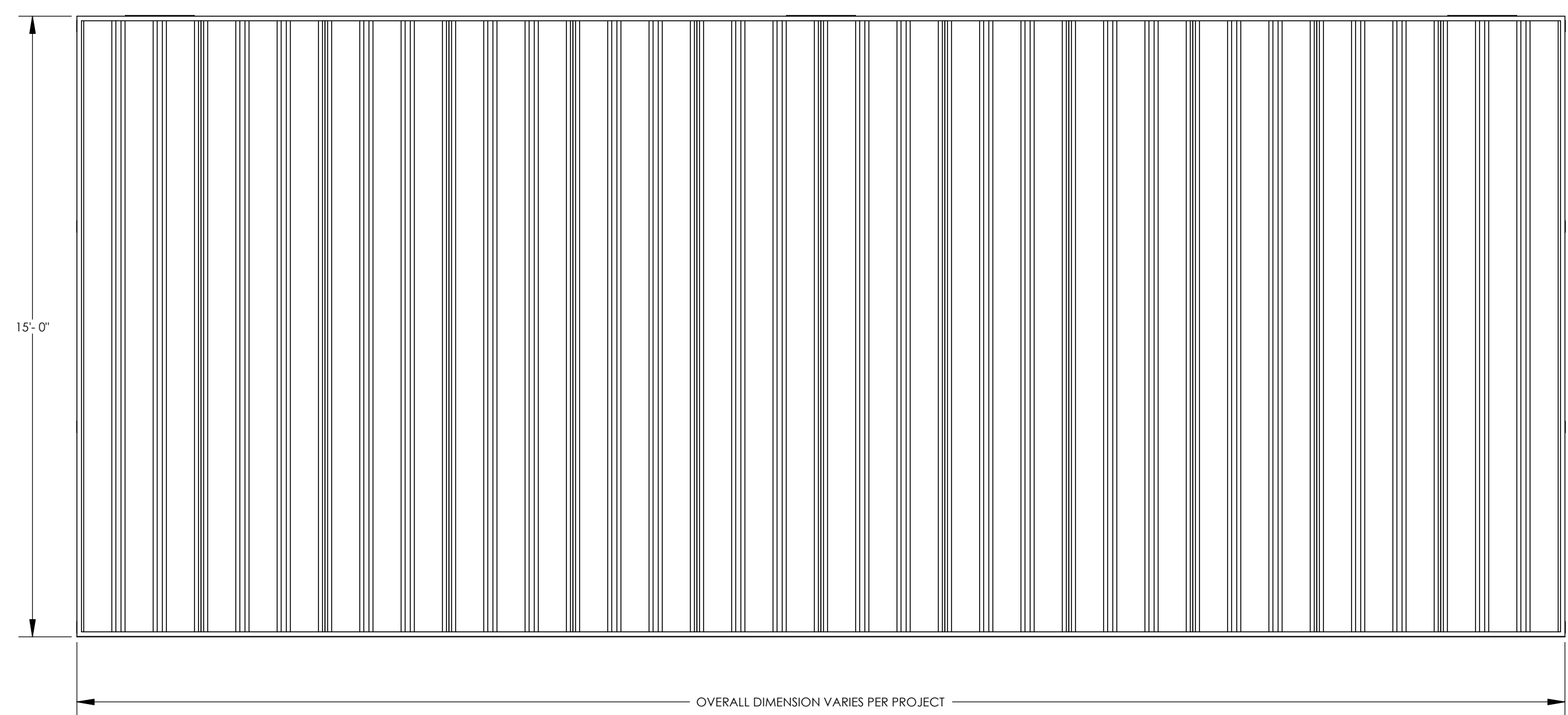
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CODE: 2022 CBC
A SEPARATE PROJECT
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**ARCHITECTURAL
VIEWS**
WALKWAY COVER - CWC 15

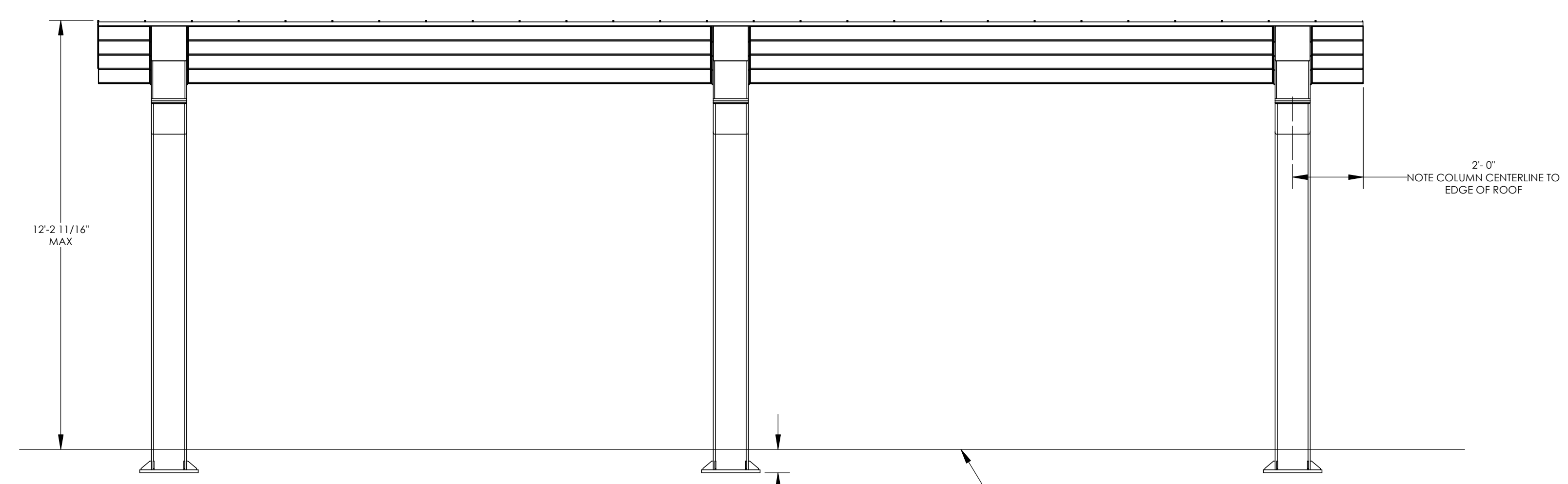
CWC5.1



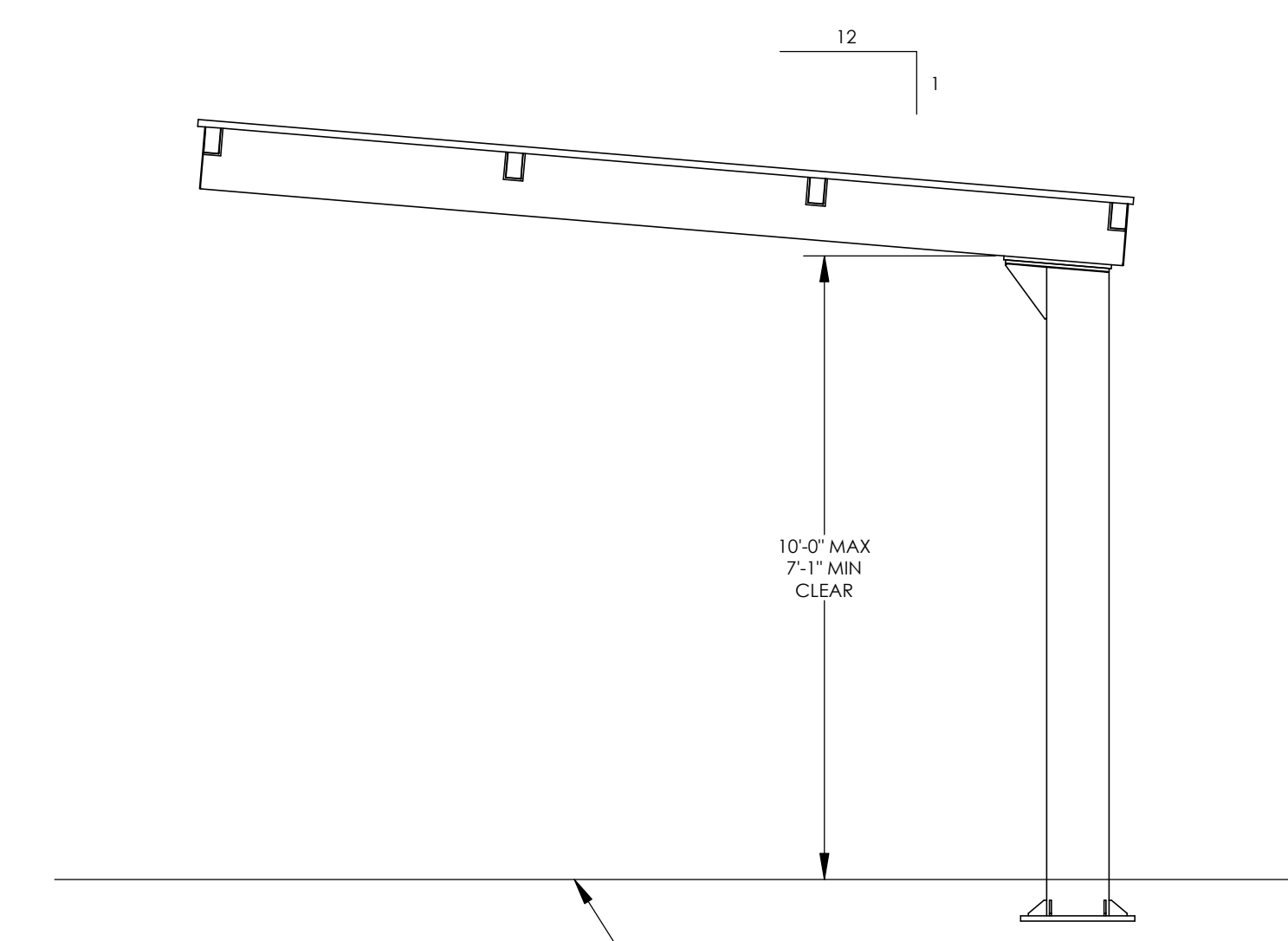
ISOMETRIC VIEW
SCALE: 3/8" = 1'-0"



PLAN VIEW
SCALE: 3/8" = 1'-0"



FRONT ELEVATION
SCALE: 3/8" = 1'-0"



SIDE ELEVATION
SCALE: 3/8" = 1'-0"

15'-0"

OVERALL DIMENSION VARIES PER PROJECT

12'-2 11/16"
MAX

2'-0"
NOTE COLUMN CENTERLINE TO
EDGE OF ROOF

FINISH GRADE
(ASSUMED AT CONSTANT
ELEVATION UNLESS
OTHERWISE NOTED)

SEE DETAIL 2
SHEET CWC2.2/CWC2.3

10'-0" MAX
7'-1" MIN
CLEAR

FINISH GRADE
(ASSUMED AT CONSTANT
ELEVATION UNLESS
OTHERWISE NOTED)

MULTI-RIB NOTES:

THE DETAILS SHOWN ARE SUGGESTIONS OR GUIDELINES ON HOW TO ERECT THE SYSTEMS. THE INFORMATION SHOWN IS ACCURATE, BUT IT IS NOT INTENDED TO COVER ALL INSTANCES. BUILDING REQUIREMENTS, DESIGNS OR CODES. THE DETAILS MAY REQUIRE CHANGES OR REVISIONS DUE TO FIELD CONDITIONS.

IT SHALL BE THE RESPONSIBILITY OF THE ERECTOR TO ENSURE THAT THE DETAILS MEET PARTICULAR BUILDING REQUIREMENTS AND TO ASSURE ADEQUATE WATER TIGHTNESS.

THE ERECTOR SHOULD THOROUGHLY FAMILIARIZE HIMSELF/HERSELF WITH ALL ERECTION INSTRUCTIONS BEFORE STARTING WORK.

THE PANELS SHOULD BE INSTALLED PLUMB, STRAIGHT, AND ACCURATELY TO THE ADJACENT WORK.

FLASHING AND TRIM SHALL BE INSTALLED TRUE, AND IN PROPER ALIGNMENT, WITH ANY EXPOSED FASTENERS EQUALLY SPACED FOR THE BEST APPEARANCE.

SEALANT SHALL BE FIELD APPLIED ON DRY, CLEAN SURFACES. SOME FIELD CUTTING AND FITTING OF PANELS AND FLASHING IS TO BE EXPECTED BY THE ERECTOR AND MINOR FIELD CORRECTIONS ARE A PART OF NORMAL ERECTION WORK.

WORKMANSHIP SHALL BE OF THE BEST INDUSTRY STANDARDS AND INSTALLATION SHALL BE PERFORMED BY EXPERIENCED METAL CRAFTSMEN.

METAL SHAVINGS FROM DRILLING OR INSTALLATION OF ROOF FASTENERS MUST BE CAREFULLY REMOVED FROM THE ROOF BY BRUSHING OR SWEEPING AT THE END OF EACH DAY DURING INSTALLATION. SHAVINGS LEFT ON THE ROOF WILL QUICKLY RUST AND STAIN THE ROOF FINISH.

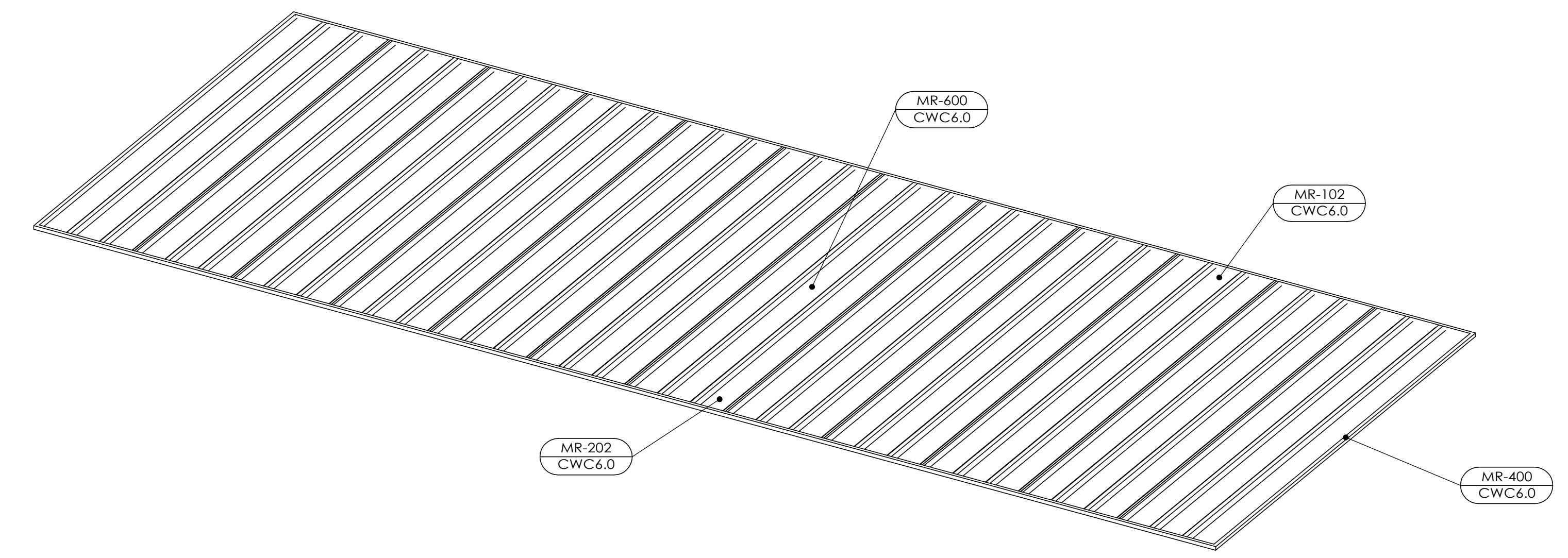
COVER ACCESS HOLES WITH GRACE ICE AND WATER SHIELD BEFORE ATTACHING ROOF DECK.

METAL ROOFING PRODUCT AND INSTALLATION SHALL MEET ALL REQUIREMENTS OF UL 580.

4035 P. AZA, GOLDEN RIDGE DRIVE
SUITE B
CHAPARRAL, CA 91791
951 877 6016

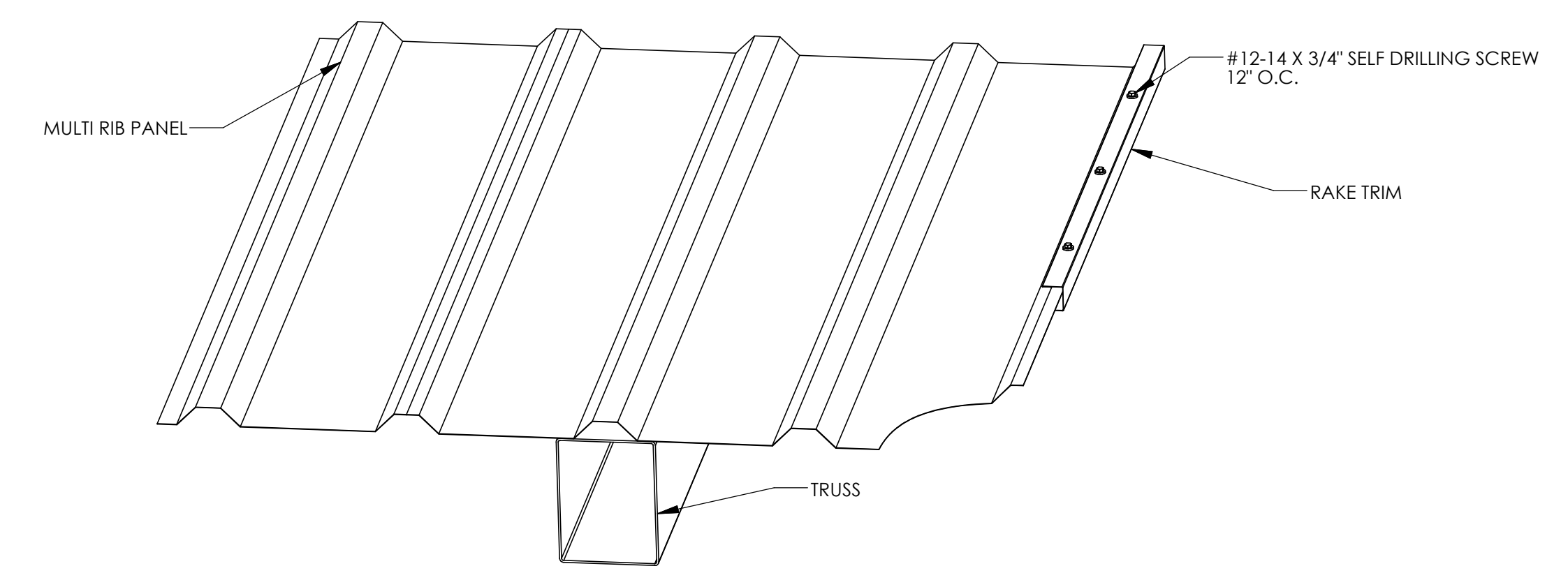


poligon
PORTER
A HANOVER COMPANY



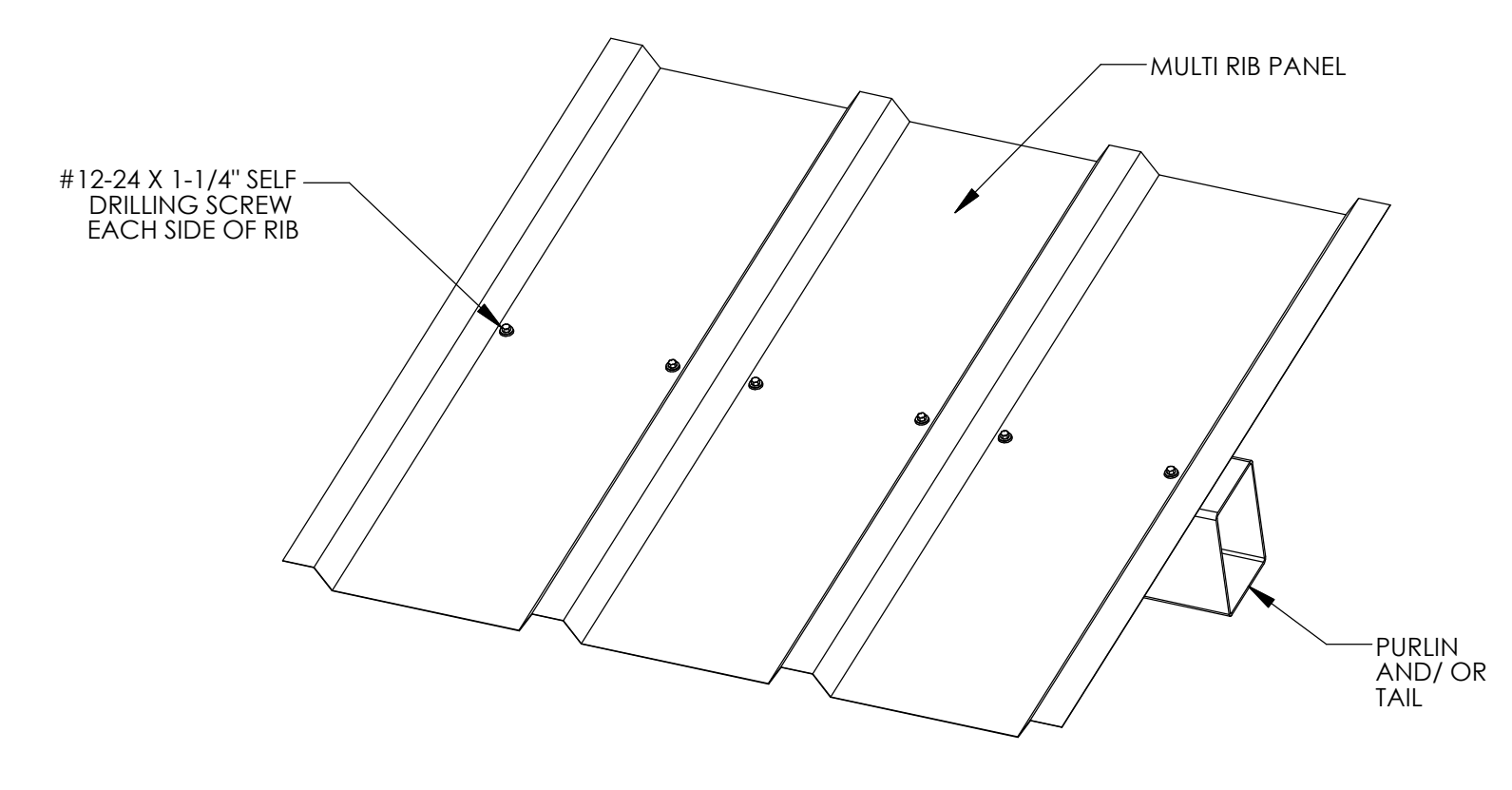
ISOMETRIC VIEW
SCALE: 3/8" = 1'-0"

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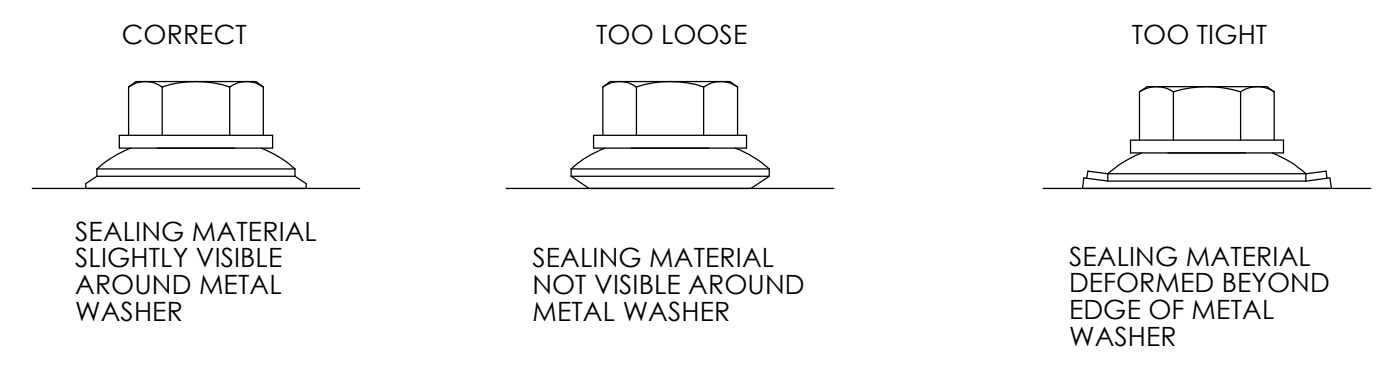
RAKE DETAIL

MR-400



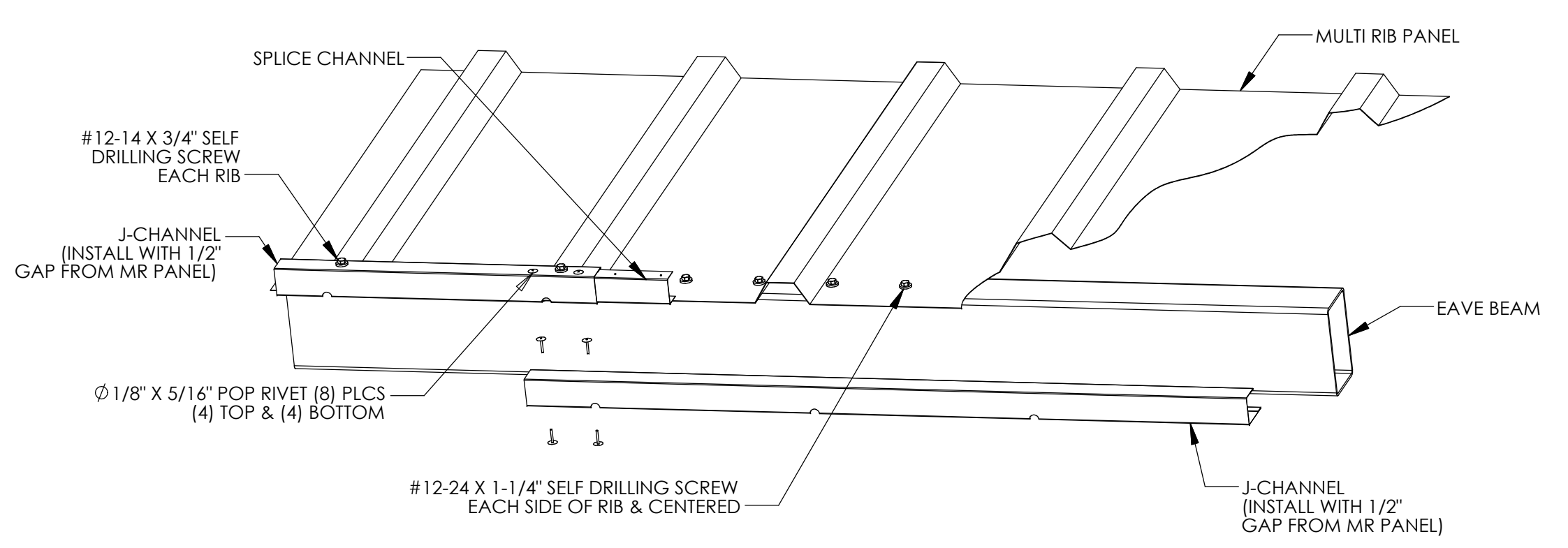
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MR-600



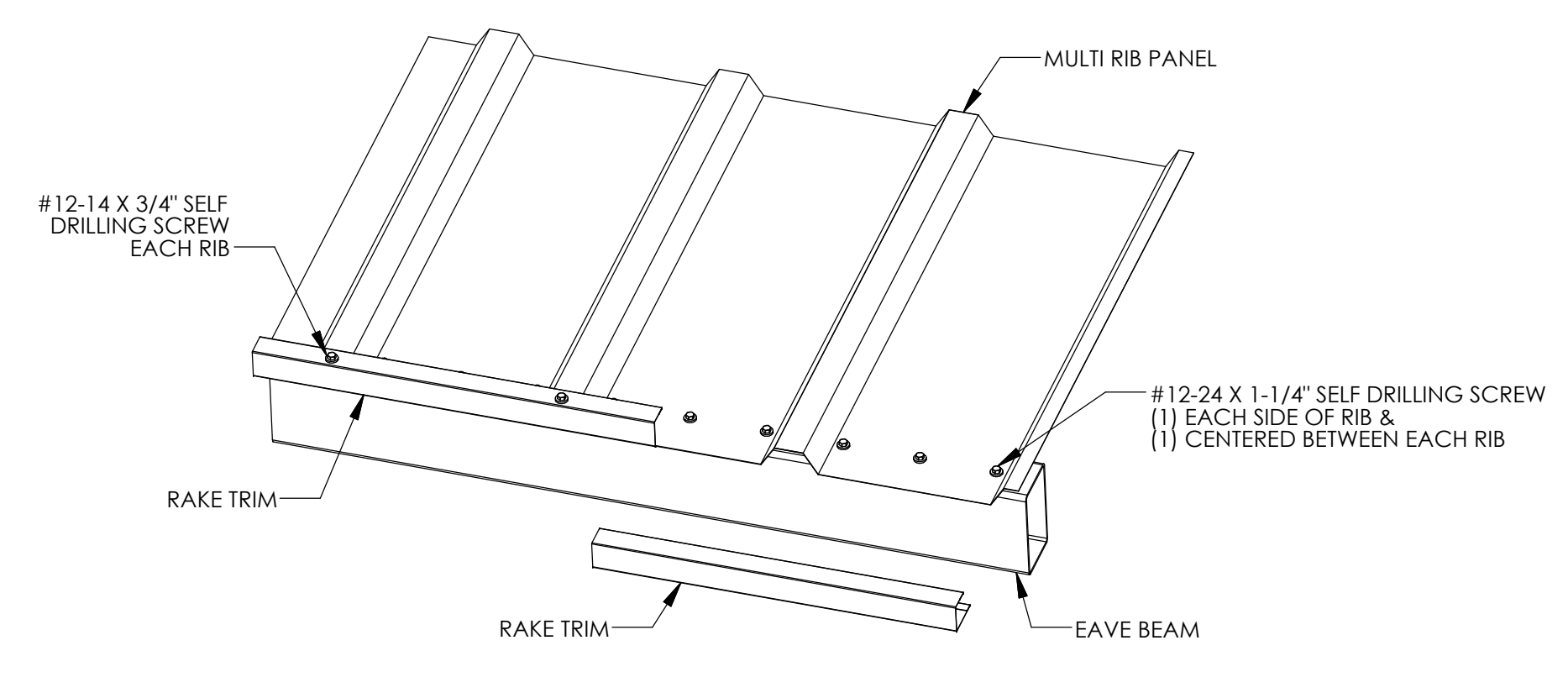
ROOF FASTENER TIGHTENING

MR-950



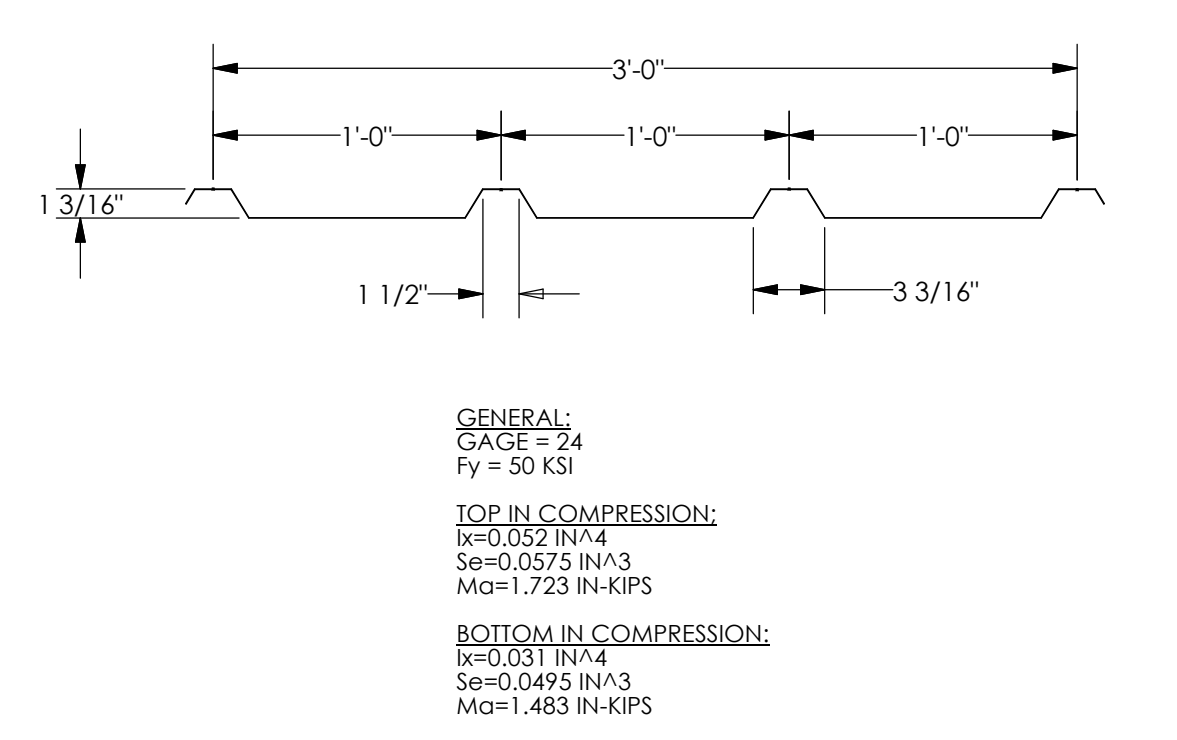
EAVE DETAIL

MR-102



HIGHSIDE DETAIL (VIEWING FROM ABOVE)

MR-202



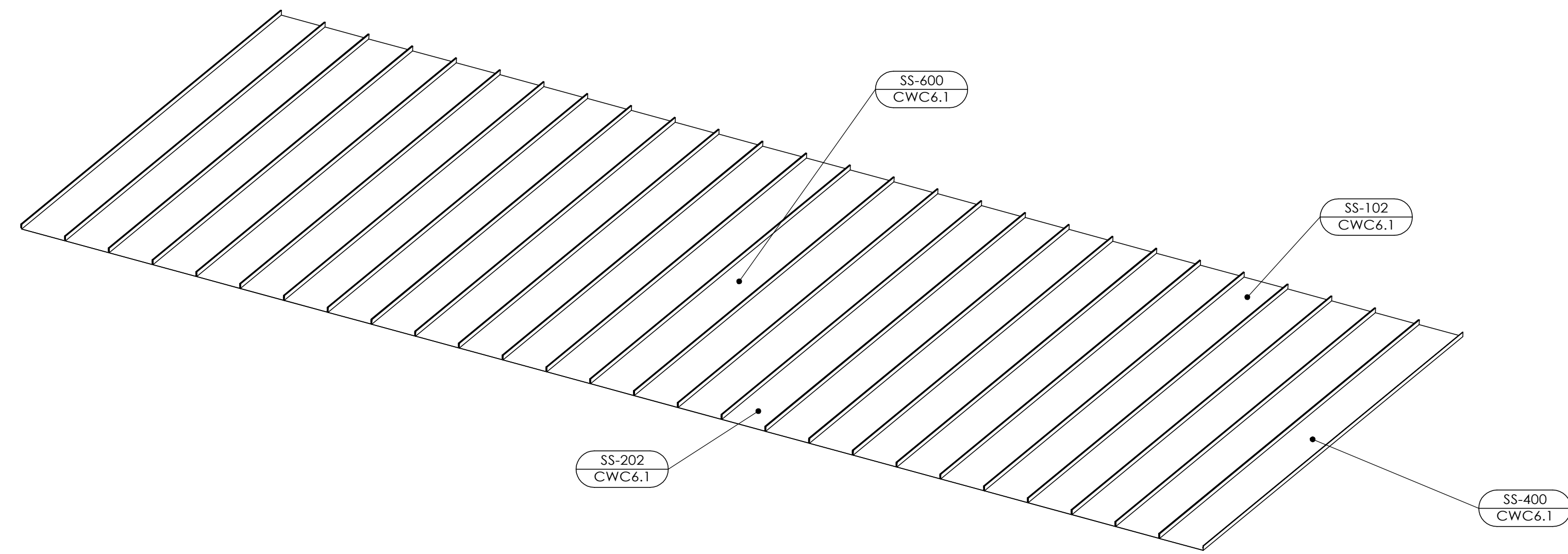
MR ROOF DECK SECTION PROPERTIES

MR-951

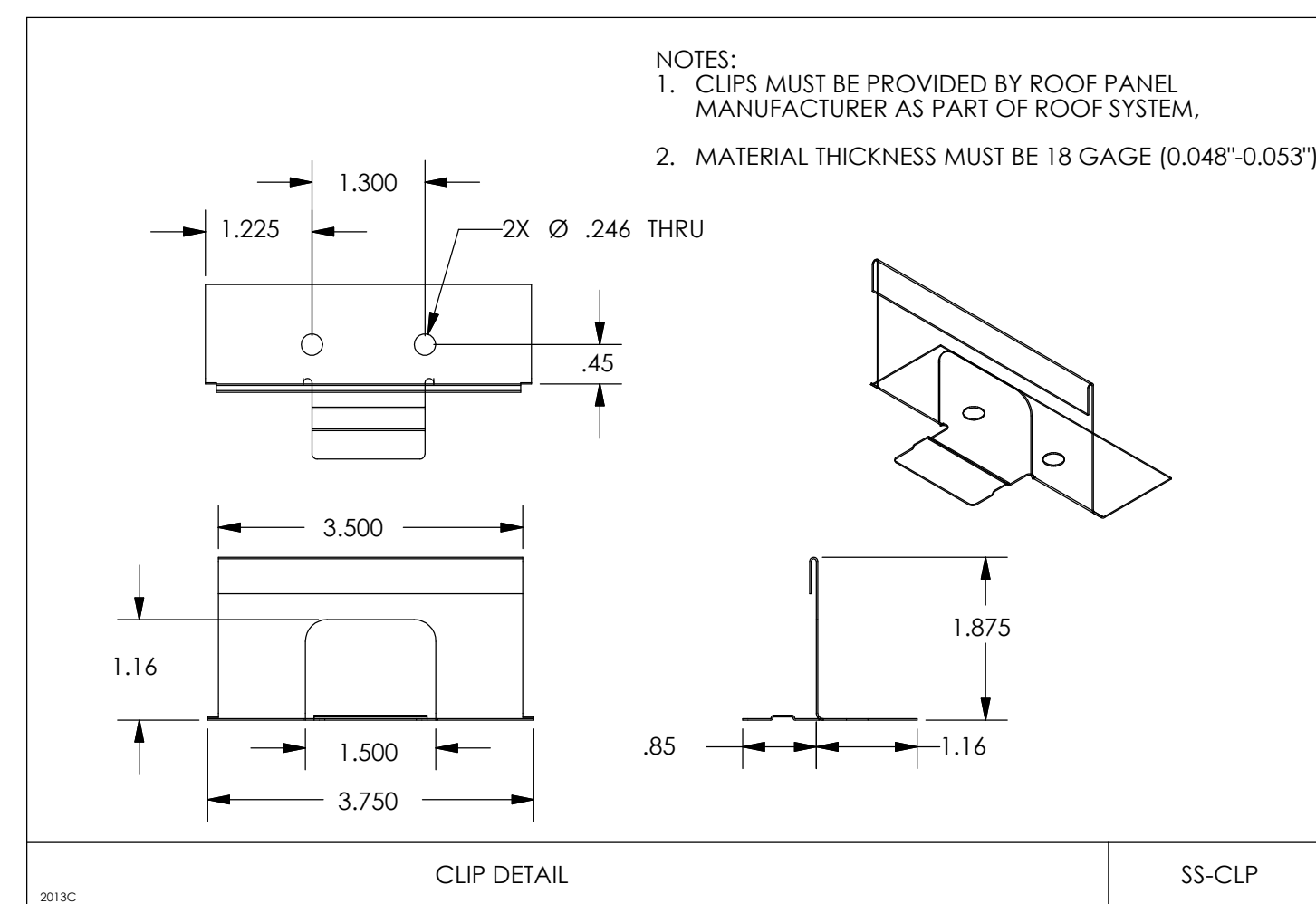
PRE-CHECK (PC)
DOCUMENT
CODE: 2022 CBC
A SEPARATE PROJECT APPLICATION FOR CONSTRUCTION IS REQUIRED.

ROOF CONNECTION DETAILS
WALKWAY COVER - CWC

CWC6.0



ISOMETRIC VIEW
SCALE: 3/16" = 1'-0"



STANDING SEAM INSTALLATION NOTES:

THE DETAILS SHOWN ARE SUGGESTIONS OR GUIDELINES ON HOW TO ERECT THE SYSTEMS. THE INFORMATION SHOWN IS ACCURATE, BUT IT IS NOT INTENDED TO COVER ALL INSTANCES. BUILDING REQUIREMENTS, DESIGNS OR CODES. THE DETAILS MAY REQUIRE CHANGES OR REVISIONS DUE TO FIELD CONDITIONS.

IT SHALL BE THE RESPONSIBILITY OF THE ERECTOR TO ENSURE THAT THE DETAILS MEET PARTICULAR BUILDING REQUIREMENTS AND TO ASSURE ADEQUATE WATER TIGHTNESS.

THE ERECTOR SHOULD THOROUGHLY FAMILIARIZE HIMSELF/HERSELF WITH ALL ERECTION INSTRUCTIONS BEFORE STARTING WORK.

THE PANELS SHOULD BE INSTALLED PLUMB, STRAIGHT, AND ACCURATELY TO THE ADJACENT WORK.

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WORKMANSHIP SHALL BE OF THE BEST INDUSTRY STANDARDS AND INSTALLATION SHALL BE PERFORMED BY EXPERIENCED METAL CRAFTSMEN.

METAL SHAVINGS FROM DRILLING OR INSTALLATION OF ROOF FASTENERS MUST BE CAREFULLY REMOVED FROM THE ROOF BY BRUSHING OR SWEEPING AT THE END OF EACH DAY DURING INSTALLATION. SHAVINGS LEFT ON THE ROOF WILL QUICKLY RUST AND STAIN THE ROOF FINISH.

COVER ACCESS HOLES WITH GRACE ICE AND WATER SHIELD BEFORE ATTACHING ROOF DECK.

METAL ROOFING PRODUCT AND INSTALLATION SHALL MEET ALL REQUIREMENTS OF ICC-ES REPORT ESL-1082.

STATE APPROVALS-SITE

1035 P. AZA, GOLDEN GATE DISTRICT
SUITE 111
CAMERON PARK, CA 95822
530.877.6016

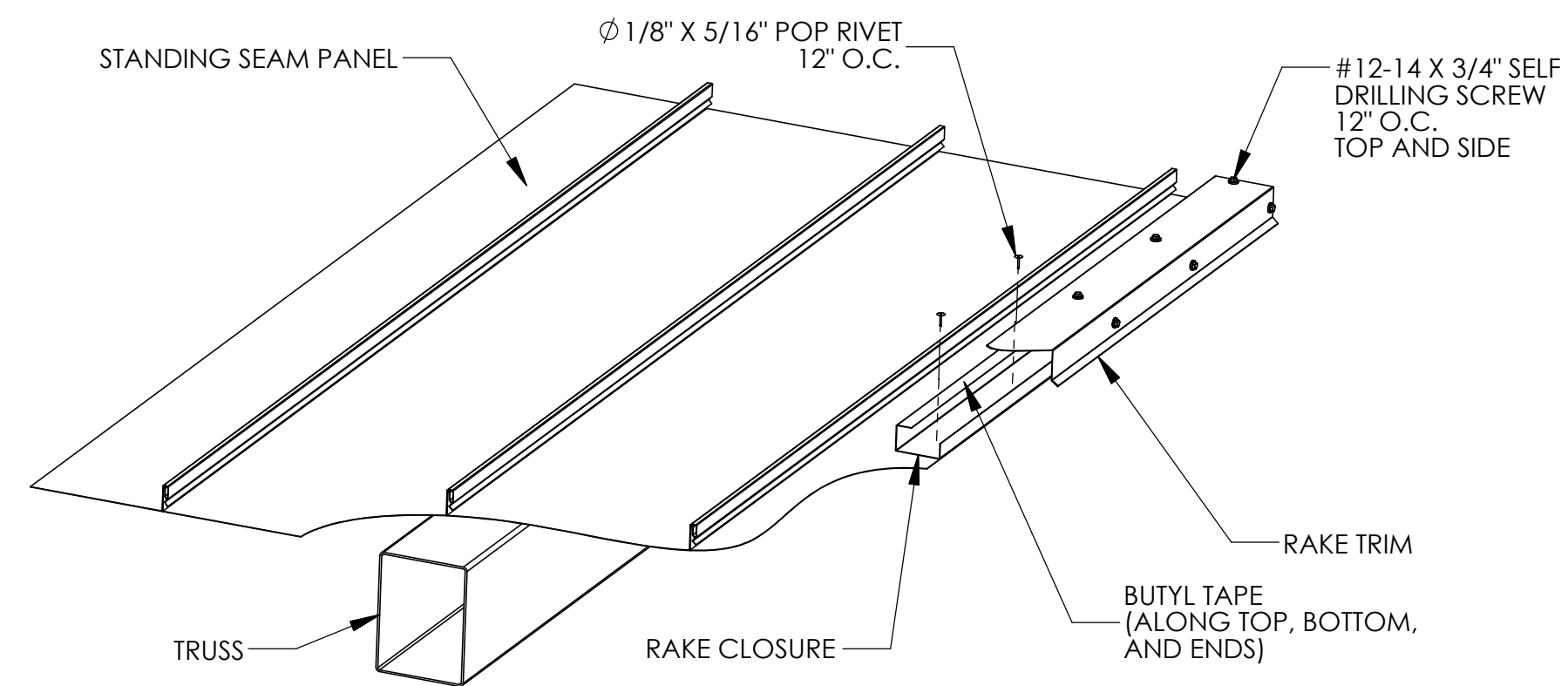


poligon
PORTER
STRUCTURAL FABRICATORS



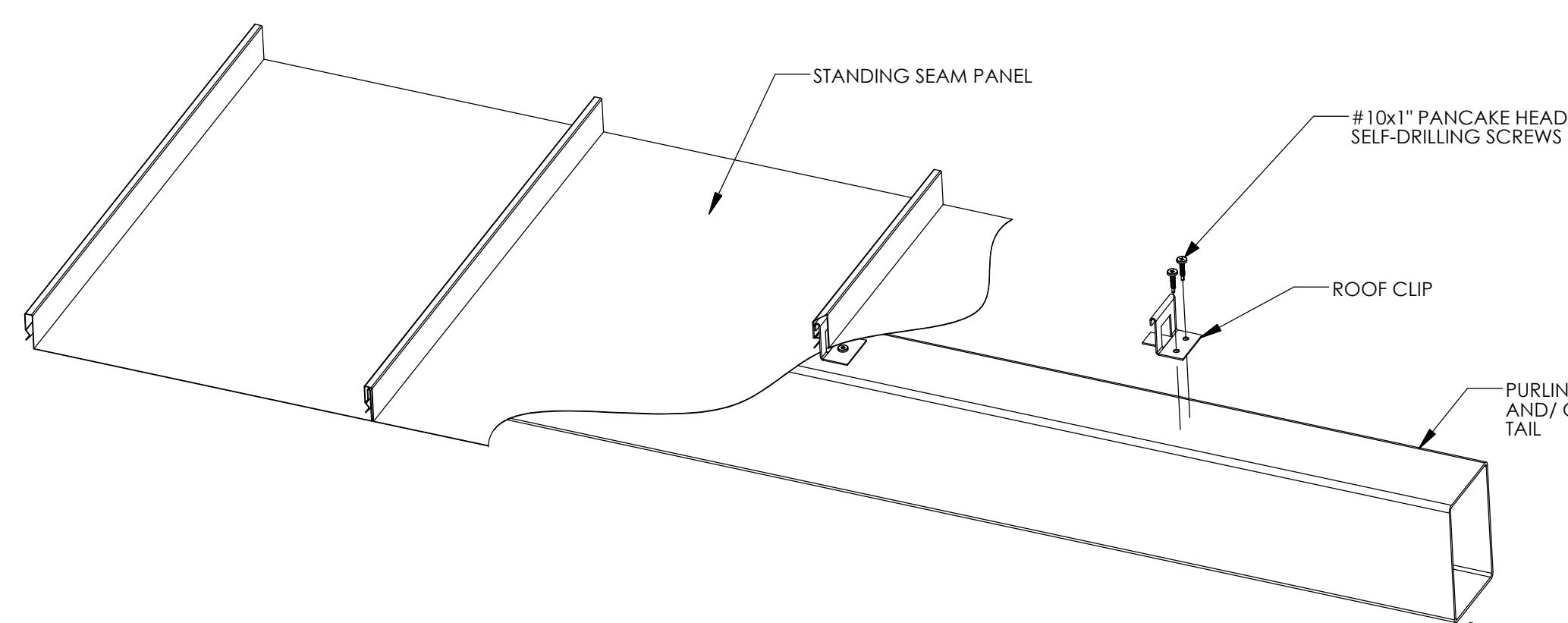
STATE APPROVALS-PC

IDENTIFICATION STAMP
DIV. OF THE STATE ARCHITECT
APP: 02-121211 PC
REVIEWED FOR
SS FLS ACS CG
DATE: 7/14/2023



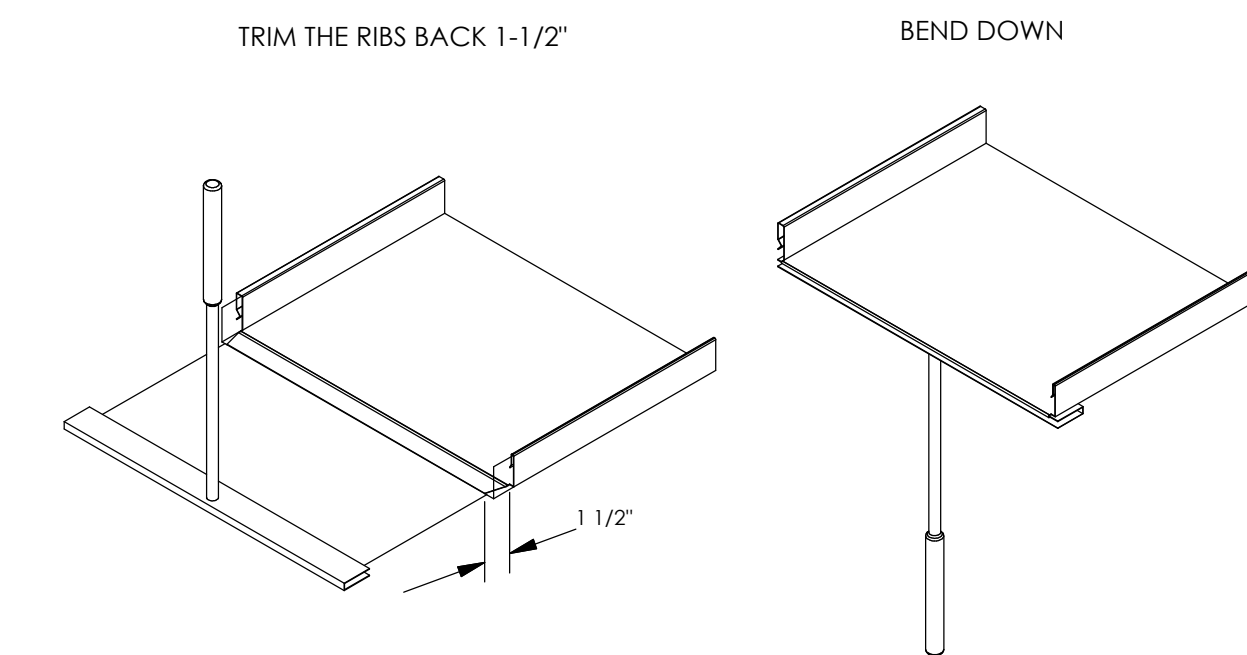
RAKE DETAIL

SS-400



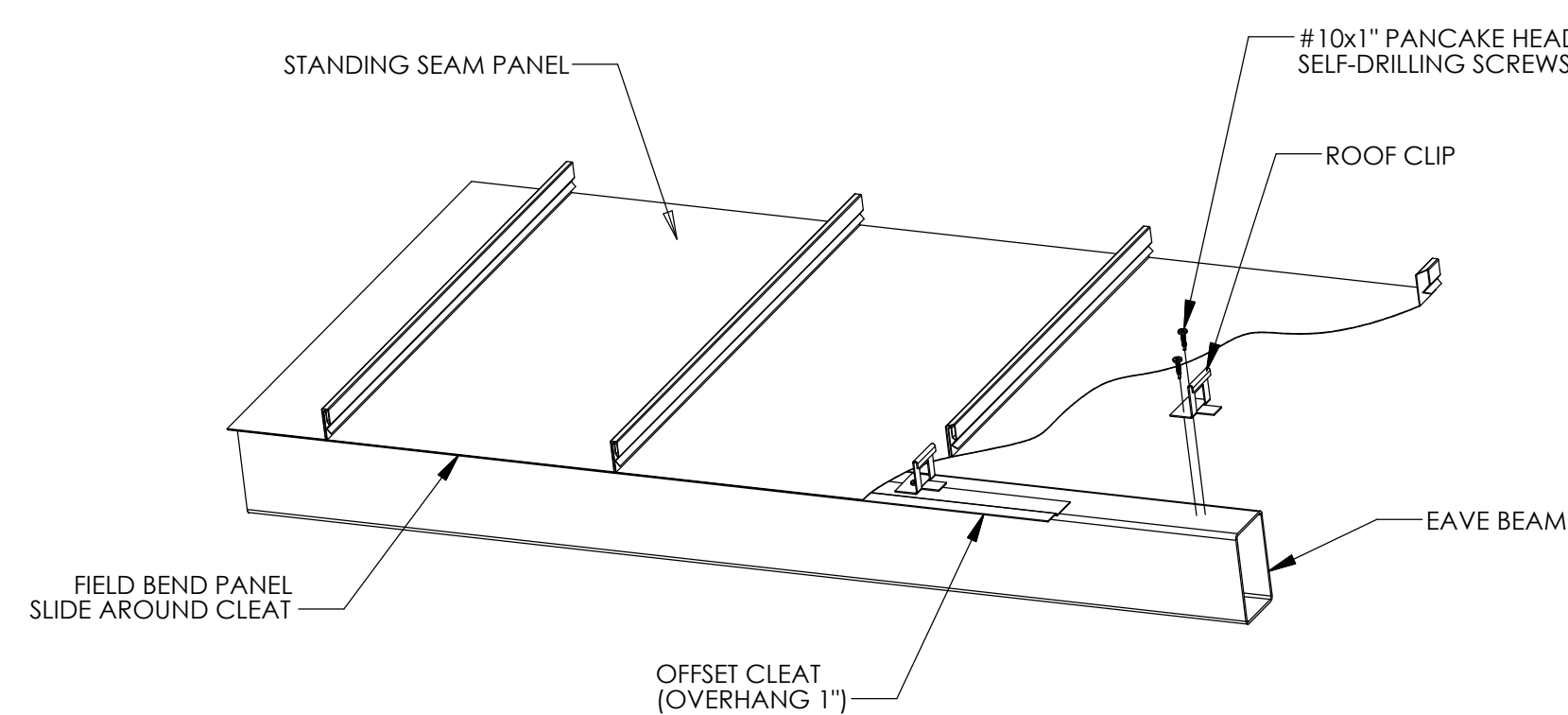
PURLIN DETAIL

SS-600



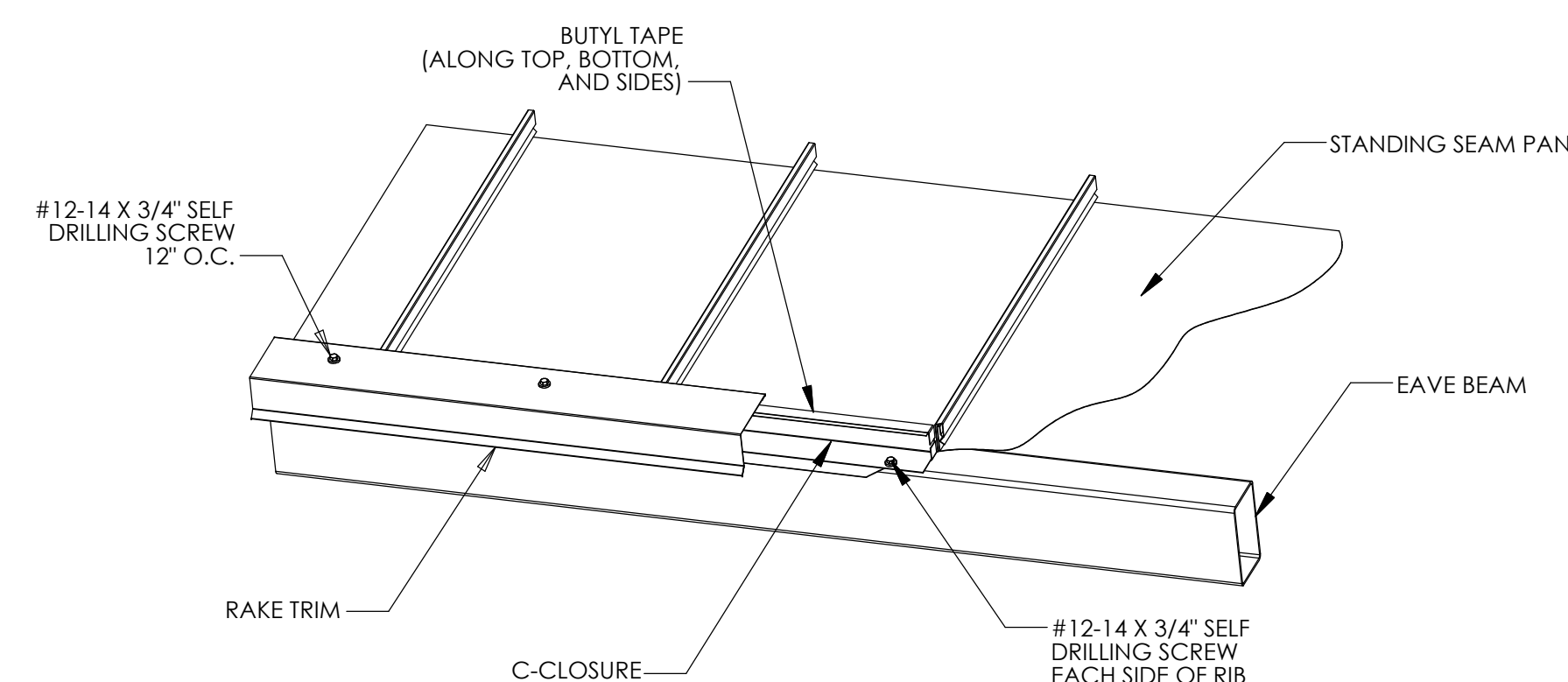
HEMMING DETAIL

SS-HEM



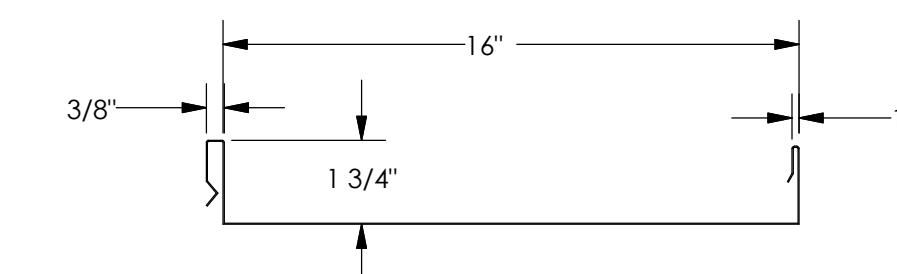
EAVE DETAIL

SS-102



HIGH SIDE DETAIL (VIEWING FROM ABOVE)

SS-202



GENERAL:
GAGE = 22
Fy = 50 KSI
TOP IN COMPRESSION:
I_x=0.1200 IN⁴
S_e=0.0803 IN³
M_x=2.405 IN-KIPS
BOTTOM IN COMPRESSION:
I_x=0.0570 IN⁴
S_e=0.0729 IN³
M_x=1.722 IN-KIPS

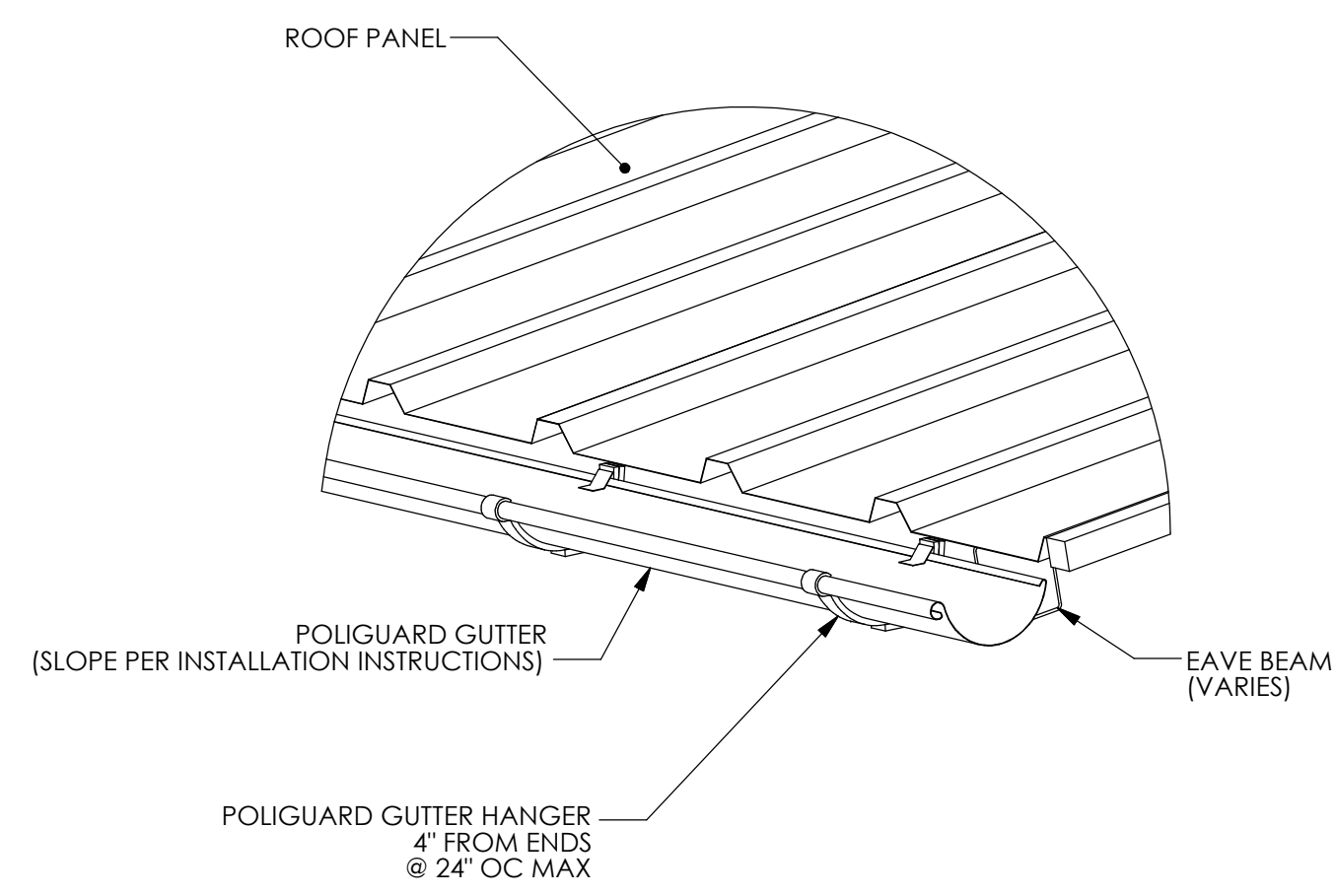
ROOF DECK SECTION PROPERTIES

SS-950

PRE-CHECK (PC)
DOCUMENT
CODE: 2022 CBC
A SEPARATE PROJECT
APPLICATION FOR
CONSTRUCTION IS REQUIRED.

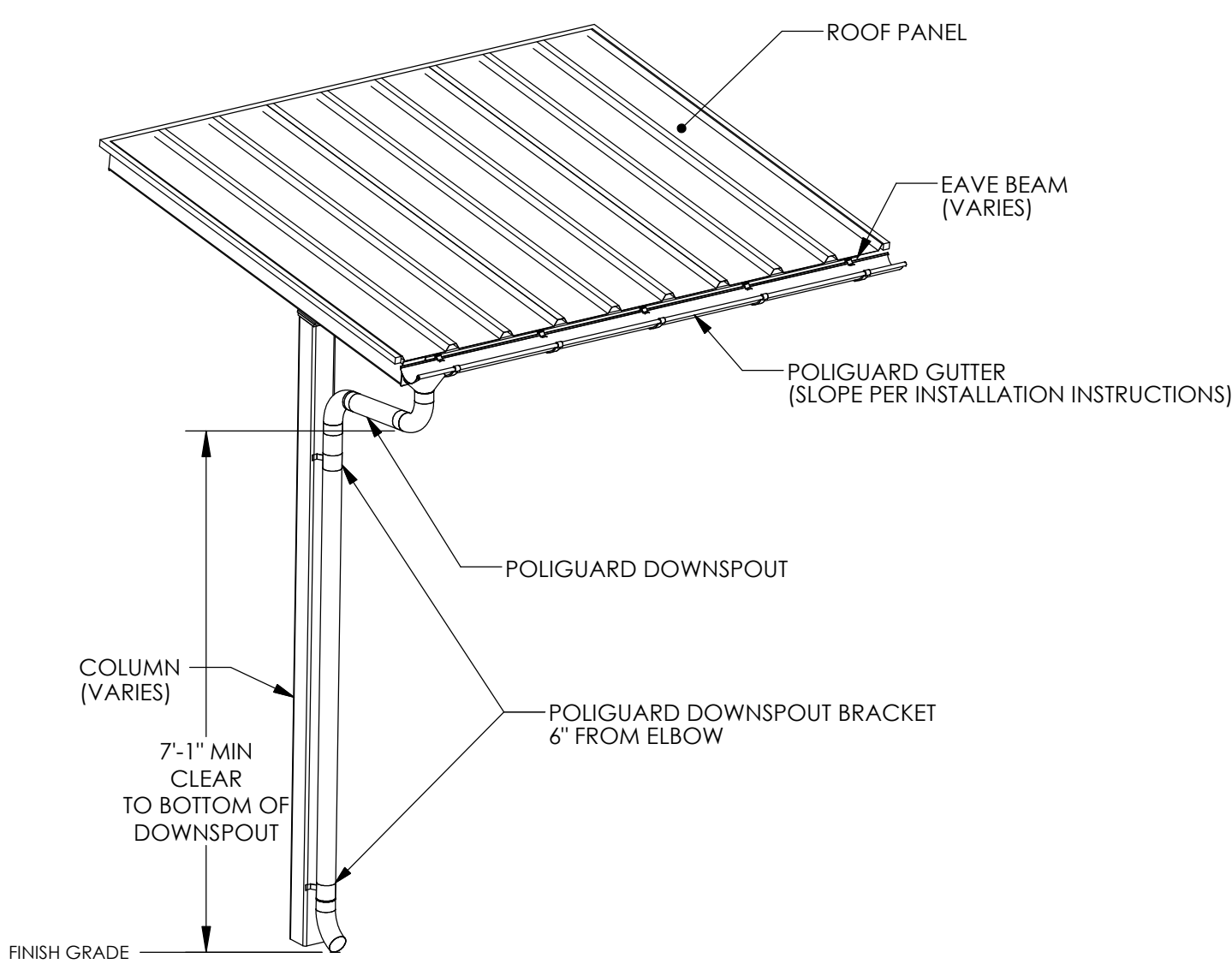
ROOF
CONNECTION
DETAILS
WALKWAY COVER - CWC

CWC6.1



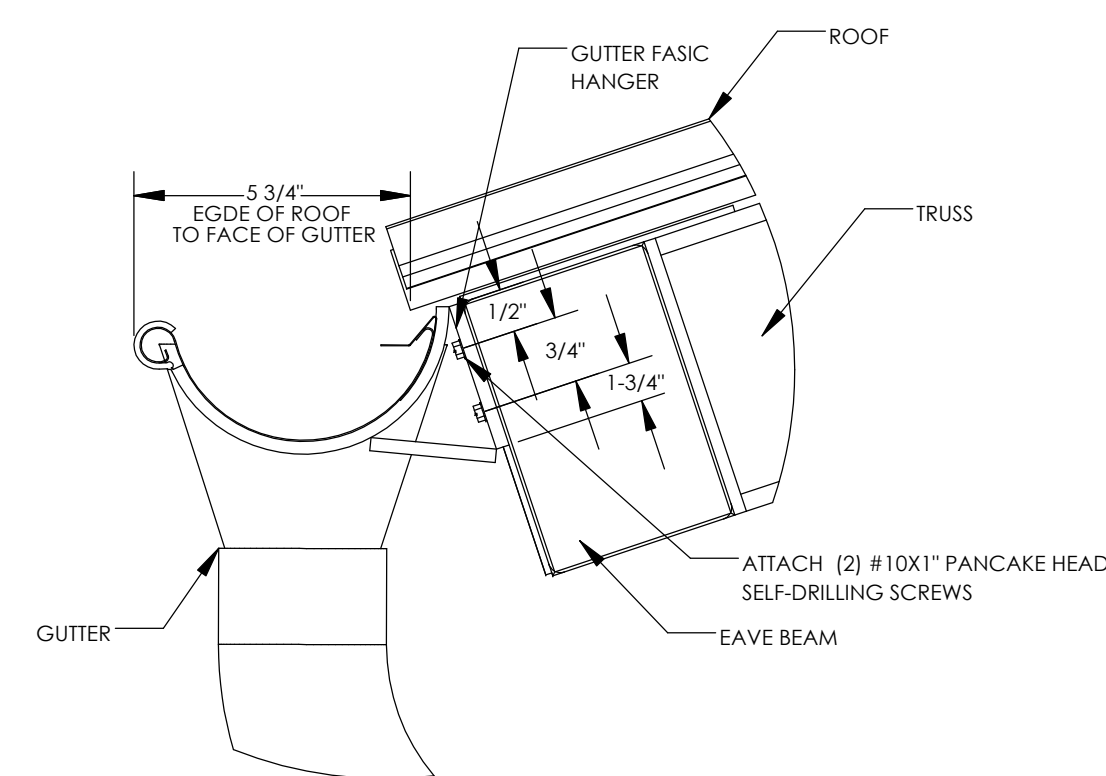
GUTTER DETAIL

GS-100



DOWNSPOUT DETAIL

GS-200

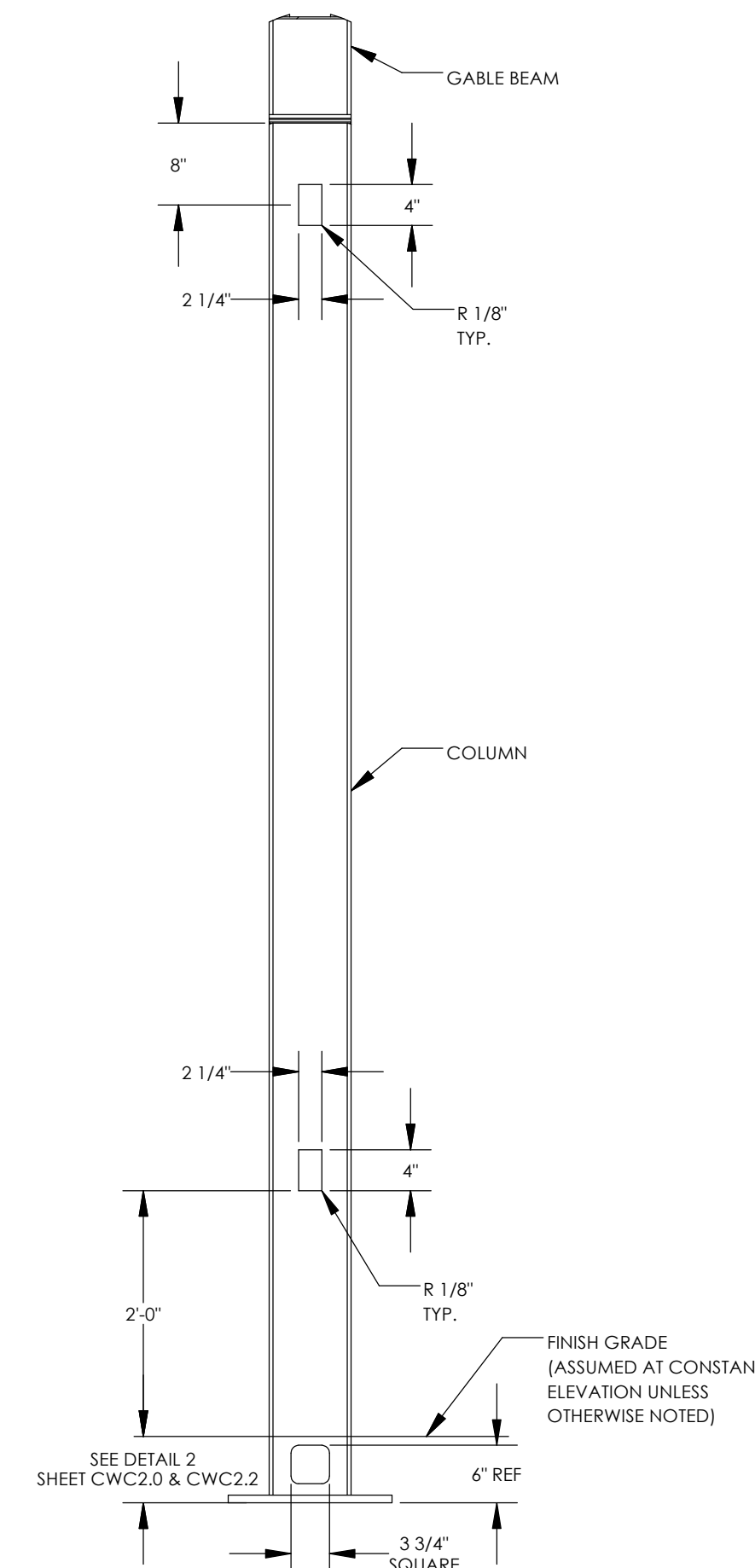


GUTTER DETAIL

GS-300

POLIGUARD GUTTER SYSTEM NOTES:

1. PREFABRICATED GUTTER SYSTEM IS ATTACHED TO THE STRUCTURE AFTER ROOF IS INSTALLED.
2. DETAILED INSTALLATION INSTRUCTIONS ARE SHIPPED WITH THE STRUCTURE.
3. DOWNSPOUTS REQUIRED AT EACH COLUMN.



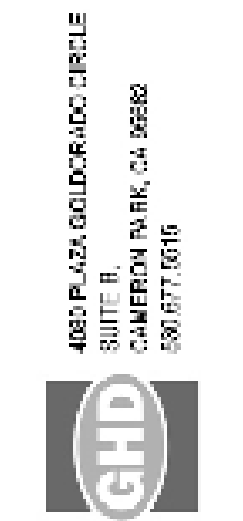
ELECTRICAL CUTOUT IN COLUMNS

EC-100

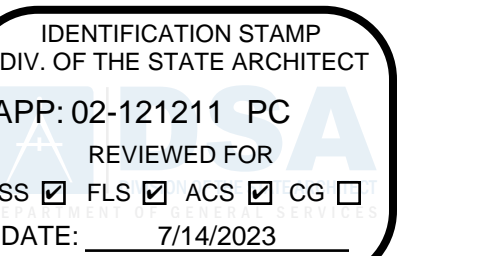
ELECTRICAL CUTOUT NOTES:

1. MAXIMUM ONE CUTOUT PERMITTED IN EACH MEMBER.
2. CUTOUTS CAN BE PLACED ON ANY SIDE OF A MEMBER.
3. CUTOUTS CAN BE PLACED ALONG MEMBERS AS INDICATED IN THE DETAILS.
4. ARCHITECTS REQUESTING CUTOUTS MUST MARKUP APPROVED PC DRAWINGS TO LOCATE CUTOUTS FOR APPROVAL AND FABRICATION.

STATE APPROVALS-SITE



STATE APPROVALS-PC



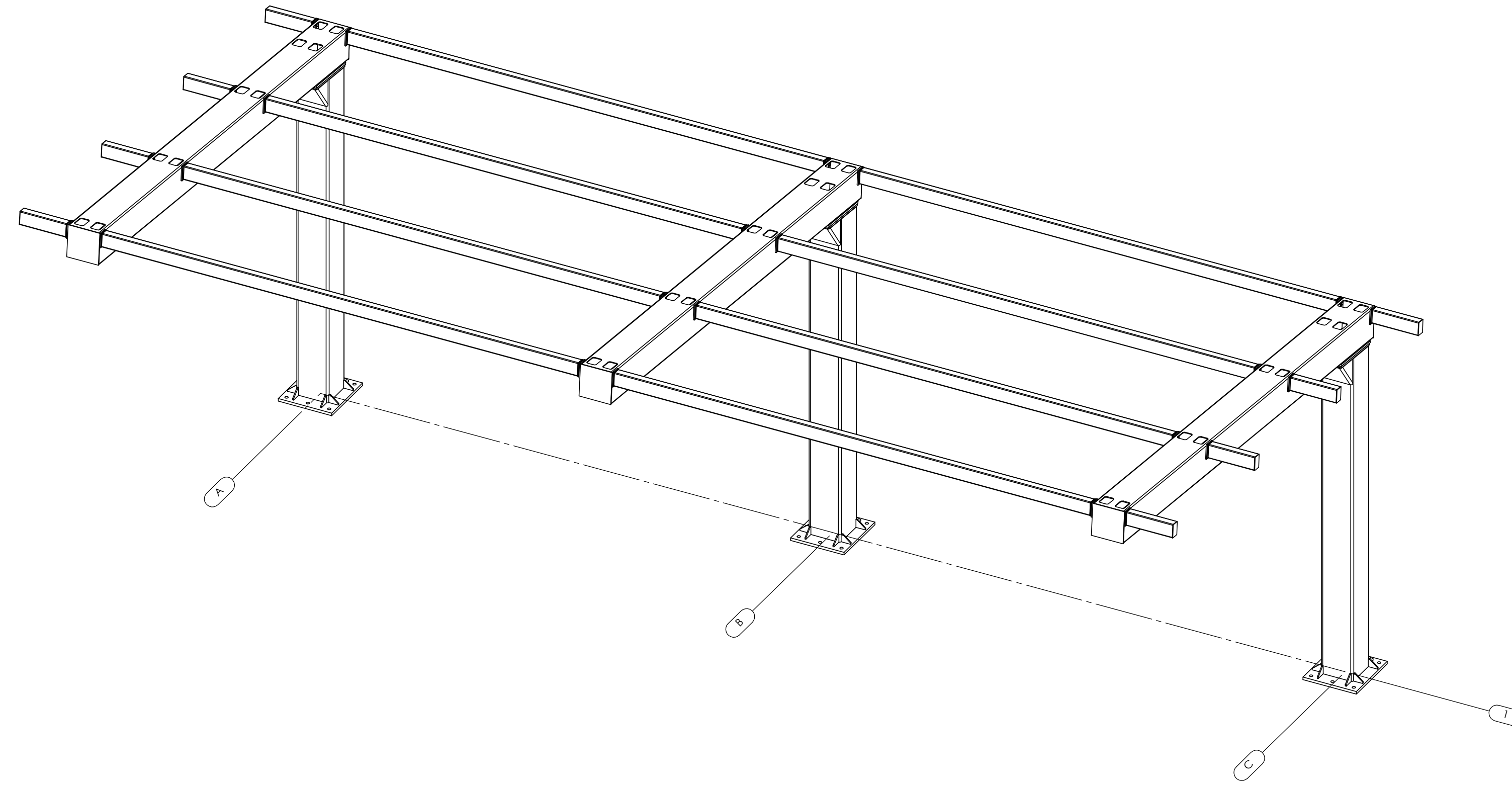
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CODE: 2022 CBC
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MISC DESIGN OPTIONS
 WALKWAY COVER - CWC

CWC7.0

ELECTRICAL CUTOUT AND ACCESS INSTRUCTIONS

- IF 'YES' IS NOT SELECTED IN STEP 2 ON ORDER FORM, THEN THIS SHEET NEED NOT BE INCLUDED IN SITE-SPECIFIC DRAWINGS
- ONLY COLUMNS ARE PERMITTED TO HAVE ELECTRICAL ACCESS
- THE COLUMN CUTOUTS ARE STATIC AND SHOWN IN THE 'MISC DESIGN OPTIONS SHEET'
- IDENTIFY THE COLUMNS WITH ELECTRICAL CUTOUTS BELOW (REFERENCE GRID LINES IN ISOMETRIC FRAME VIEW TO THE RIGHT)
- STRUCTURES MAY BE LONGER OR SHORTER THAN THE ISOMETRIC FRAME VIEW SHOWN
- IF SITE-SPECIFIC STRUCTURE HAS A DIFFERENT NUMBER OF COLUMNS THAN ISOMETRIC SHOWN, REFERENCE COLUMN A1 IN THE ISOMETRIC VIEW AND CONTINUE PATTERN TO FIT SITE-SPECIFIC LAYOUT
- IF NO COLUMNS ARE IDENTIFIED, POLIGON WILL ASSUME CUTOUTS ONLY IN COLUMN A1
- CONTACT POLIGON ENGINEERING FOR SPECIAL PROJECT SPECIFIC REQUIREMENTS



ELECTRICAL CUTOUT IDENTIFICATION IN COLUMNS
SPECIFIC MEMBERS _____

EXAMPLE:

ELECTRICAL CUTOUT IDENTIFICATION IN COLUMNS
SPECIFIC MEMBERS _____ A1, B1, F1

STATE APPROVALS-SITE

4033 P. AZA, GOLDEN GATE DISTRICT
SUITE 11
CAMERON PARK, CA 95822
530.877.0016

poligon
PORTER & PARTNERS INC.

STATE APPROVALS-PC

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ELECTRICAL CUTOUTS
WALKWAY COVER - CWC

CWC7.1